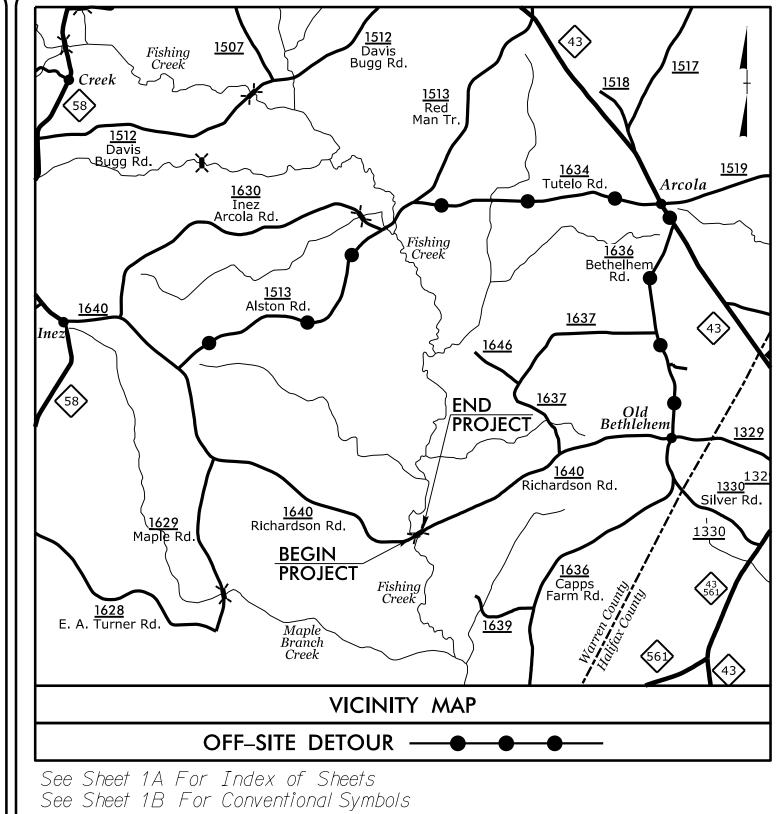
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PROJECT:17BP.5.R.77

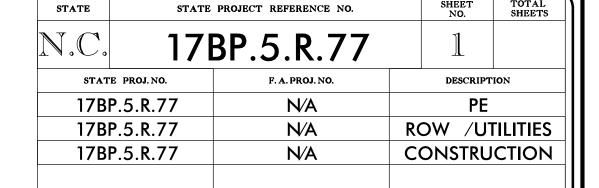


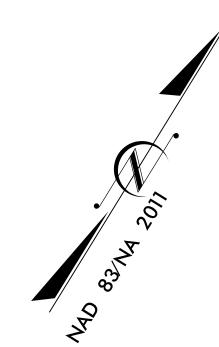
STATE OF NORTH CAROLINA
DIVISION OF HIGHWAYS

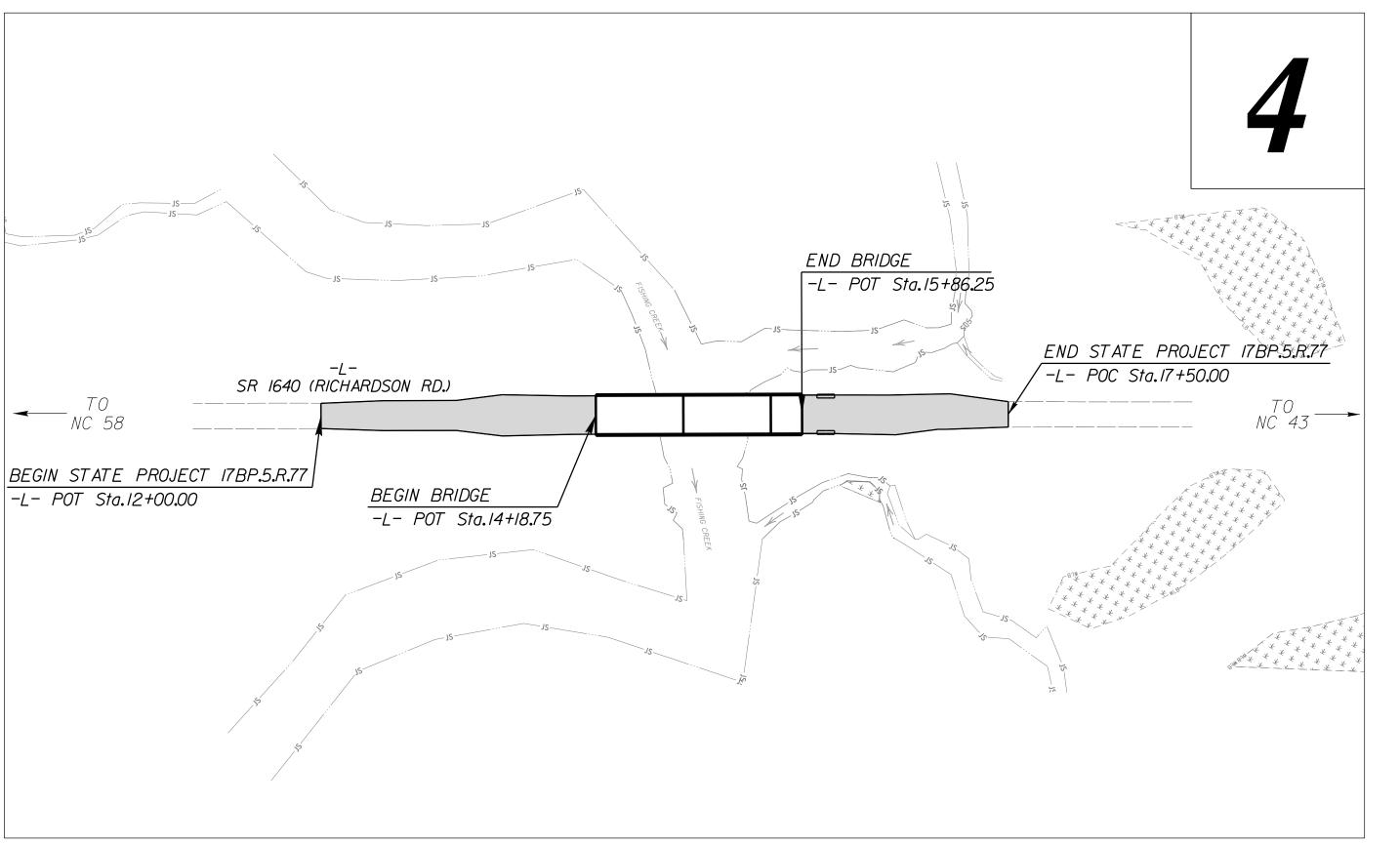
WARREN COUNTY

LOCATION: BRIDGE NO. 77 OVER FISHING CREEK ON SR 1640 (RICHARDSON RD.)

TYPE OF WORK: GRADING, DRAINAGE, PAVING, AND STRUCTURE







DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

NTRACT: D

9

 GRAPHIC SCALES

 25 0 50 100

PLANS

0 25 0 50 100

PROFILE (HORIZONTAL)

0 5 0 10 20

PROFILE (VERTICAL)

DESIGN DATA

ADT = 1,000 VPD V = 60 MPH CLASS = RURAL LOCAL

SUBREGIONAL TIER

PROJECT LENGTH

LENGTH ROADWAY STATE PROJECT 17BP.5.R.77 = 0.072 mi.

LENGTH STRUCTURES STATE PROJECT 17BP.5.R.77 = 0.032 mi.

TOTAL LENGTH STATE PROJECT 17BP.5.R.77 = 0.104 mi.

Prepared in the Offices of:

STEWART

STEWART

23 S. WEST ST, STE 1100
RALEIGH, NC 27603
T 919.380.8750

Plirm Ucense #: C-1051
www.stewartlnc.com

Plirm Ucense #: C-1051
www.stewartlnc.com

Plirm Ucense #: C-1051
www.stewartlnc.com

ANDY YOUNG, PE
PROJECT ENGINEER

RIGHT OF WAY DATE:

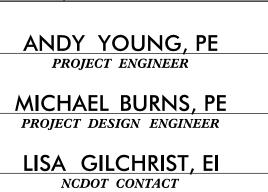
DECEMBER 23, 2017

RIGHT OF WAY

COMPLETE:

JUNE 23, 2018

LETTING DATE: SEPTEMBER 12, 2018



TOOCUSIGNED BY:

Reid B. Rolol

7074FDA3F200470...

SIGNATURE:

P.E.

7/26/2018

P.E.

DocuSigned by:

7/26/2018

P.E.

SEAL

043870

P.E.

SIGNATURE:

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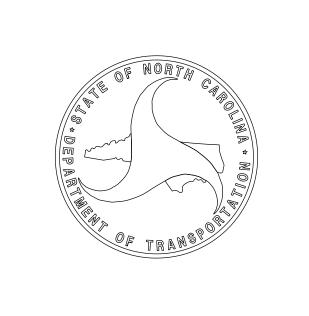
SIGNATURE:

SIGNATURE:

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SIGNATURE:

HYDRAULICS ENGINEER



PROJECT REFERENCE NO. SHEET NO. 17BP.5.R.77 /A

ROADWAY DESIGN ENGINEER 045230 Michael S. Burns, Jr.

STEWART

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INDEX OF SHEETS

SHEET NUMBER SHEET TITLE SHEET

INDEX OF SHEETS, GENERAL NOTES, AND STANDARD DRAWINGS

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PAVEMENT SCHEDULE AND TYPICAL SECTIONS 2A-1

2C-1 GUARDRAIL INSTALLATION DETAIL STRUCTURE ANCHOR UNIT DETAIL 2C-2

ROADWAY SUMMARIES 3D-1 DRAINAGE SUMMARY GEOTECHNICAL SUMMARY

PLAN SHEET PROFILE SHEET TRAFFIC MANAGEMENT PLANS PAVEMENT MARKING PLANS EC-1 THRU EC-5 EROSION CONTROL PLANS RF -1 REFORESTATION SHEET UO-1 THRU UO-2 UTILITIES BY OTHERS PLANS

CROSS-SECTION SUMMARY SHEET

X-1 THRU X-4 CROSS-SECTIONS STRUCTURE PLANS S-1 THRU S-23

X-1 A

EFF. 01-16-2018

2018 ROADWAY ENGLISH STANDARD DRAWINGS

The following Roadway Standards as appear in "Roadway Standard Drawings" Highway Design Branch -N. C. Department of Transportation – Raleigh, N. C., Dated January, 2018 are applicable to this project and by reference hereby are considered a part of these plans:

STD.NO.

DIVISION 2 - EARTHWORK

200.02 Method of Clearing - Method II 225.02 Guide for Grading Subgrade - Secondary and Local 225.04 Method of Obtaining Superelevation - Two Lane Pavement 275.01 Rock Plating DIVISION 3 - PIPE CULVERTS 300.01 Method of Pipe Installation DIVISION 4 - MAJOR STRUCTURES 422.02 Bridge Approach Fills - Type II Modified Approach Fill DIVISION 5 - SUBGRADE, BASES AND SHOULDERS 560.01 Method of Shoulder Construction – High Side of Superelevated Curve – Method I DIVISION 8 - INCIDENTALS 806.01 Concrete Right-of-Way Marker 806.02 Granite Right-of-Way Marker 840.00 Concrete Base Pad for Drainage Structures 840.25 Anchorage for Frames - Brick or Concrete or Precast 840.29 Frames and Narrow Slot Flat Grates 840.35 Traffic Bearing Grated Drop Inlet – for Cast Iron Double Frame and Grates 840.46 Traffic Bearing Precast Drainage Structure 840.66 Drainage Structure Steps 846.01 Concrete Curb, Gutter and Curb & Gutter 846.04 Drop Inlet Installation in Shoulder Berm Gutter 862.01 Guardrail Placement 862.02 Guardrail Installation (Special Detail for Sheet 6 of 8) 862.03 Structure Anchor Units (Special Detail for Type III Anchor Units Sheets 1 of 7 and 2 of 7) 876.02 Guide for Rip Rap at Pipe Outlets

GENERAL NOTES:

2018 SPECIFICATIONS EFFECTIVE: 01-16-2018 REVISED:

GRADE LINE: GRADING AND SURFACING:

> THE GRADE LINES SHOWN DENOTE THE FINISHED ELEVATION OF THE PROPOSED SURFACING AT GRADE POINTS SHOWN ON THE TYPICAL SECTIONS. GRADE LINES MAY BE ADJUSTED AT THEIR BEGINNING AND ENDING AND AT STRUCTURES AS DIRECTED BY THE ENGINEER IN ORDER TO SECURE A PROPER TIE-IN.

CLEARING:

CLEARING ON THIS PROJECT SHALL BE PERFORMED TO THE LIMITS ESTABLISHED BY METHOD III.

SUPERELEVATION:

ALL CURVES ON THIS PROJECT SHALL BE SUPERELEVATED IN ACCORDANCE WITH STD. NO. 225.04 USING THE RATE OF SUPERELEVATION AND RUNOFF SHOWN ON THE PLANS. SUPERELEVATION IS TO BE REVOLVED ABOUT THE GRADE POINTS SHOWN ON THE TYPICAL SECTIONS.

SHOULDER CONSTRUCTION:

ASPHALT, EARTH, AND CONCRETE SHOULDER CONSTRUCTION ON THE HIGH SIDE OF

SUPERELEVATED CURVES SHALL BE IN ACCORDANCE WITH STD. NO. 560.01

GUARDRAIL:

THE GUARDRAIL LOCATIONS SHOWN ON THE PLANS MAY BE ADJUSTED DURING CONSTRUCTION AS DIRECTED BY THE ENGINEER. THE CONTRACTOR SHOULD CONSULT

TEMPORARY SHORING:

SHORING REQUIRED FOR THE MAINTENANCE OF TRAFFIC WILL BE PAID FOR AS "EXTRA WORK" IN ACCORDANCE WITH SECTION 104-7.

END BENTS:

THE SURVEYOR SHALL CHECK THE STRUCTURE END BENT PLANS, DETAILS, AND CROSS-SECTION PRIOR TO SETTING OF THE SLOPE STAKES FOR THE EMBANKMENT OR EXCAVATION APPROACHING A BRIDGE.

UTILITIES:

UTILITY OWNERS ON THIS PROJECT ARE

CenturyLink - Telecommunications

Halifax Electric - Power

Warren County Public Utilities - Water

ANY RELOCATION OF EXISTING UTILITIES WILL BE ACCOMPLISHED BY OTHERS.

RIGHT-OF-WAY MARKERS:

ALL RIGHT-OF-WAY MARKERS ON THIS PROJECT SHALL BE PLACED BY CONTRACT.

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STATE OF NORTH CAROLINA, DIVISION OF HIGHWAYS

PROJECT REFERENCE NO.	SHEET NO.
17BP.5.R.77	IB

CONVENTIONAL PLAN SHEET SYMBOLS **BOUNDARIES AND PROPERTY:** Note: Not to Scale *S.U.E. = Subsurface Utility Engineering

State Line		RAILROADS:	Scale 'S
		Standard Gauge	CSX TRANSPORTATION
Soonly Line		RR Signal Milepost	MILEPOST 35
Township Line		Switch —	
City Line		RR Abandoned	SWITCH
Reservation Line		RR Dismantled	
Property Line			
Existing Iron Pin		DICUT OF WAY & DDOIECT C	ONTROI.
Computed Property Corner	×	RIGHT OF WAY & PROJECT C	ONTROL:
Property Monument	ECM	Secondary Horiz and Vert Control Point	
Parcel/Sequence Number ————————————————————————————————————		Primary Horiz Control Point	•
Existing Fence Line	×××_	Primary Horiz and Vert Control Point	•
Proposed Woven Wire Fence		Exist Permanent Easment Pin and Cap	\diamondsuit
Proposed Chain Link Fence		New Permanent Easement Pin and Cap ——	♦
Proposed Barbed Wire Fence		Vertical Benchmark	
Existing Wetland Boundary		Existing Right of Way Marker	
Proposed Wetland Boundary		Existing Right of Way Line	
•	EAB	New Right of Way Line	$\frac{R}{W}$
inig Endangered / ammar beendary	EPB	New Right of Way Line with Pin and Cap—	$\frac{\mathbb{R}}{\mathbb{R}}$
ixiomig Endangered Flam Boomaary	FAB FAB	14ew Right of Way Line with Till and Cap—	W
Existing Historic Property Boundary	——————————————————————————————————————	New Right of Way Line with Concrete or Granite R/W Marker	$ \stackrel{R}{\longrightarrow}$ $\stackrel{R}{\longrightarrow}$
Known Contamination Area: Soil		New Control of Access Line with	
Potential Contamination Area: Soil		Concrete C/A Marker	-
Known Contamination Area: Water	J•• w J••	Existing Control of Access	(\bar{C})
Potential Contamination Area: Water		New Control of Access	
Contaminated Site: Known or Potential —		Existing Easement Line ——————	—— F ——
BUILDINGS AND OTHER CUI	LTURE:	New Temporary Construction Easement –	F
Gas Pump Vent or U/G Tank Cap ———	O	New Temporary Drainage Easement ——	TDE
Sign ————————————————————————————————————	<u> </u>	New Permanent Drainage Easement —	
Well ———————————————————————————————————			PDE
Small Mine	<u></u>	New Permanent Drainage / Utility Easement	
Foundation —		New Permanent Utility Easement ———	PUE
Area Outline		New Temporary Utility Easement ————	IOL
Cemetery —		New Aerial Utility Easement —————	——— AUE———
•			
Jonania	<u> </u>	ROADS AND RELATED FEATUR	
School ———————————————————————————————————	+	Existing Edge of Pavement	
Church —		Existing Curb	
Dam ————————————————————————————————————		Proposed Slope Stakes Cut	
HYDROLOGY:		Proposed Slope Stakes Fill	F
Stream or Body of Water ——————		Proposed Curb Ramp	
Hydro, Pool or Reservoir ————————————————————————————————————		Existing Metal Guardrail	ттттт
Iurisdictional Stream	Js	Proposed Guardrail —————	
Buffer Zone 1 ———————————————————————————————————	——————————————————————————————————————	Existing Cable Guiderail	
Buffer Zone 2 ———————————————————————————————————		Proposed Cable Guiderail	
Flow Arrow —		Equality Symbol	
Disappearing Stream ————————————————————————————————————	>	Pavement Removal ————————————————————————————————————	
Spring ————————————————————————————————————	_ 0		
Wetland	<u> </u>	VEGETATION:	<u> </u>
Proposed Lateral, Tail, Head Ditch ————	≥ FIOW	Single Tree	- ÷
False Sump ————————————————————————————————————	← FLOW ← FLOW	Single Shrub	_
-	~~		

Hodgo ———————————————————————————————————	
Woods Line	
Orchard —	
Vineyard —	
	Villeyal a
EXISTING STRUCTURES:	
MAJOR: Pridge Turnel of Pay Culvert	CONC
Bridge, Tunnel or Box Culvert Bridge Wing Wall, Head Wall and End Wall	
MINOR:) conc "" (
Head and End Wall	CONC HW
Pipe Culvert	
Footbridge —————	>
Drainage Box: Catch Basin, DI or JB	СВ
Paved Ditch Gutter	
Storm Sewer Manhole	(\$)
Storm Sewer —	s
UTILITIES:	
POWER: Existing Power Pole	_
Proposed Power Pole	_
Existing Joint Use Pole	1
Proposed Joint Use Pole	-
Power Manhole	
Power Line Tower	
Power Transformer	
U/G Power Cable Hand Hole	
H-Frame Pole	
U/G Power Line LOS B (S.U.E.*)	
U/G Power Line LOS C (S.U.E.*)	
U/G Power Line LOS D (S.U.E.*)	
TELEPHONE:	
Existing Telephone Pole	
Proposed Telephone Pole	
Telephone Manhole	
Telephone Pedestal	
Telephone Cell Tower	
U/G Telephone Cable Hand Hole	
U/G Telephone Cable LOS B (S.U.E.*)	
U/G Telephone Cable LOS C (S.U.E.*)	
U/G Telephone Cable LOS D (S.U.E.*)	
U/G Telephone Conduit LOS B (S.U.E.*)	TC
U/G Telephone Conduit LOS C (S.U.E.*)	- — — TC— — —
U/G Telephone Conduit LOS D (S.U.E.*)	TC

U/G Fiber Optics Cable LOS B (S.U.E.*) -----

U/G Fiber Optics Cable LOS C (S.U.E.*) ------

U/G Fiber Optics Cable LOS D (S.U.E.*)—— T FO ———

WATER:	
Water Manhole	- W
Water Meter	- 🔾
Water Valve	- ⊗
Water Hydrant	
U/G Water Line LOS B (S.U.E*)	
U/G Water Line LOS C (S.U.E*)	
U/G Water Line LOS D (S.U.E*)	
Above Ground Water Line	
TV:	
TV Pedestal ————————————————————————————————————	
TV Tower	-
U/G TV Cable Hand Hole	– H _H
U/G TV Cable LOS B (S.U.E.*)	TV
U/G TV Cable LOS C (S.U.E.*)	
U/G TV Cable LOS D (S.U.E.*)	TV
U/G Fiber Optic Cable LOS B (S.U.E.*)	TV FO— —
U/G Fiber Optic Cable LOS C (S.U.E.*)	— — — TV FO— — —
U/G Fiber Optic Cable LOS D (S.U.E.*)	TV F0
GAS:	
Gas Valve	- 🔷
Gas Meter ———————————————————————————————————	-
U/G Gas Line LOS B (S.U.E.*)	
U/G Gas Line LOS C (S.U.E.*)	
U/G Gas Line LOS D (S.U.E.*)	
Above Ground Gas Line	
SANITARY SEWER:	
Sanitary Sewer Manhole	-
Sanitary Sewer Cleanout	
U/G Sanitary Sewer Line ————————————————————————————————————	ss
Above Ground Sanitary Sewer ————	A/G Sanitary Sewer
SS Forced Main Line LOS B (S.U.E.*)	
SS Forced Main Line LOS C (S.U.E.*)	FSS
SS Forced Main Line LOS D (S.U.E.*)	FSS
AMCCELLANIEGUIC	
MISCELLANEOUS:	
Utility Pole ————————————————————————————————————	-
Utility Pole with Base ————————————————————————————————————	
Utility Located Object ————————————————————————————————————	
Utility Traffic Signal Box	
Utility Unknown U/G Line LOS B (S.U.E.*)	
U/G Tank; Water, Gas, Oil	
Underground Storage Tank, Approx. Loc. ——	
A/G Tank; Water, Gas, Oil	
Geoenvironmental Boring	•
U/G Test Hole LOS A (S.U.E.*)	•
Abandoned According to Utility Records —	
End of Information ————————————————————————————————————	- E.O.I.

PROJECT REFERENCE NO. SHEET NO. 92–0077 SURVEY CONTROL SHEET 92-0077 Location and Surveys W/EXISTING CENTERLINE ALIGNMENTS PRIOR TO CONSTRUCTION 920077-1 N= 907243.2920 E= 2285612.5390 ELEV= 230.28' BM-2 N= 907684 E=2286944 BM-I 920077-3 ELEV=166.13' N= 907445 N= 908132.0770 E= 2286608 E= 2287815.6710 ELEV= 166.85′ 920077-2 ELEV= 172.51 N= 907290.3320 E= 2286405.5410 ELEV= 178.86′ **→** TO NC 58 **→** TO NC 43 **→** <u>RICHARDSON ROAD (SR 1640)</u> DATUM DESCRIPTION THE LOCALIZED COORDINATE SYSTEM DEVELOPED FOR THIS PROJECT IS BASED ON THE STATE PLANE COORDINATES ESTABLISHED BY NCDOT FOR MONUMENT "920077-1" WITH NAD 83/NA 2011 STATE PLANE GRID COORDINATES OF NORTHING: 907243.292(ft) EASTING: 2285612.539(ft) ELEVATION: 230.28(ft) THE AVERAGE COMBINED GRID FACTOR USED ON THIS PROJECT (GROUND TO GRID) IS: 1.00001726 THE N.C. LAMBERT GRID BEARING AND LOCALIZED HORIZONTAL GROUND DISTANCE FROM "920077-1" TO -L- STATION IS NOTES: ALL LINEAR DIMENSIONS ARE LOCALIZED HORIZONTAL DISTANCES I. IF FURTHER INFORMATION REGARDING PROJECT CONTROL VERTICAL DATUM USED IS NAVD 88 IS NEEDED, PLEASE CONTACT THE LOCATION AND SURVEYS UNIT. NOTE: DRAWING NOT TO SCALE 2. PROJECT CONTROL WAS ESTABLISHED USING GNSS, THE GLOBAL NAVIGATION SATELLITE SYSTEM.

//24/2018 ...\Proj\920077_1s_1c USER: iroemer SURVEY CONTROL SHEET 92-0077

PROJECT REFERENCE NO. SHEET NO.

92–0077

1C–2

Location and Surveys

W/EXISTING CENTERLINE ALIGNMENTS PRIOR TO CONSTRUCTION

BASELINE

BL 	POINT	DESC.	NORTH	EAST	ELEVATION
	1	920077 - 1	907243.2920	2285612.5390	230.28′
	2	920077-2	907290.3320	2286405.5410	178.86′
	100	BL - 100	907706.0910	2287147.1280	170.18′
	3	920077-3	908132.0770	2287815.6710	172.51′

BM1 ELEVATION = 166.85'

N 907445 E 2286608

RR SPIKE IN 24" GUM

BM2 ELEVATION = 166.13'

N 907684 E 2286944

NOTES:

I. IF FURTHER INFORMATION REGARDING PROJECT CONTROL

IS NEEDED, PLEASE CONTACT THE LOCATION AND SURVEYS UNIT.

SURVEY CONTROL SHEET 92-0077

PROJECT REFERENCE NO. SHEET NO.

92–0077
1C–3

Location and Surveys

W/EXISTING CENTERLINE ALIGNMENTS PRIOR TO CONSTRUCTION

EL									
POINT	N		BEARING	DIST	DELTA			T	R
PC	907409.664	2286642.374							
CURVE			N 58°39′37.2" E	152.39	ØØ°58′48.Ø"(LT)	ØØ°38′35.2"	152.39	76.20	89Ø9.33
PT	907488.922	2286772.528							
LINE			N 58°10′13.2" E	437.49					
PC	907719.655	2287144.232							
CURVE			N 58°16′26.6" E	20.75	ØØ°12′26.9"(RT)	Ø1°ØØ′ØØ.Ø"	20.75	10.37	5729.58
PT	907730.565	2287161.88Ø							
LINE			N 58°19′57.3" E	365.97					
POT	907922.694	2287473.359							

NOTES:

I. IF FURTHER INFORMATION REGARDING PROJECT CONTROL

IS NEEDED, PLEASE CONTACT THE LOCATION AND SURVEYS UNIT.

PROPOSED	ALIGNMENT	CONTROL	SHEET	92-0077

PROJECT REFERENCE NO. SHEET NO.

92–0077 1D–1

Location and Surveys

TYPE	STATION	NORTH	EAST
PC	10+00.00	907409.6640	2286642.3745
PT	11+52.39	907488.9223	2286772.5284
PC	15+89.88	907719.6550	2287144.2322
PRC	16+10.63	907730.5653	2287161.8797
PT	19+76.60	907922.6945	2287473.3595

NOTES:

I. IF FURTHER INFORMATION REGARDING PROJECT CONTROL

IS NEEDED, PLEASE CONTACT THE LOCATION AND SURVEYS UNIT.

PROP. APPROX. 3" ASPHALT CONCRETE SURFACE COURSE, TYPE S9.5B, AT AN AVERAGE RATE OF 165 LBS. PER SQ. YD. IN EACH OF TWO LAYERS.

PAVEMENT SCHEDULE

C2	PROP. VAR. DEPTH ASPHALT CONCRETE SURFACE COURSE, TYPE S9.5B, AT AN AVERAGE RATE OF 110 LBS. PER SQ. YD. PER 1" DEPTH. TO BE PLACED IN LAYERS NOT TO EXCEED 2" IN DEPTH.

PROP. APPROX. 4" ASPHALT CONCRETE INTERMEDIATE COURSE,
TYPE I19.0C, AT AN AVERAGE RATE OF 456 LBS. PER SQ. YD.

PROP. VAR. DEPTH ASPHALT CONCRETE INTERMEDIATE COURSE,
TYPE I19.0C, AT AN AVERAGE RATE OF 114 LBS. PER SQ. YD. PER 1"
DEPTH, TO BE PLACED IN LAYERS NOT LESS THAN 2½" IN DEPTH OR
GREATER THAN 4" IN DEPTH.

PROP. APPROX. 4" ASPHALT CONCRETE BASE COURSE, TYPE B25.0C, AT AN AVERAGE RATE OF 456 LBS. PER SQ. YD.

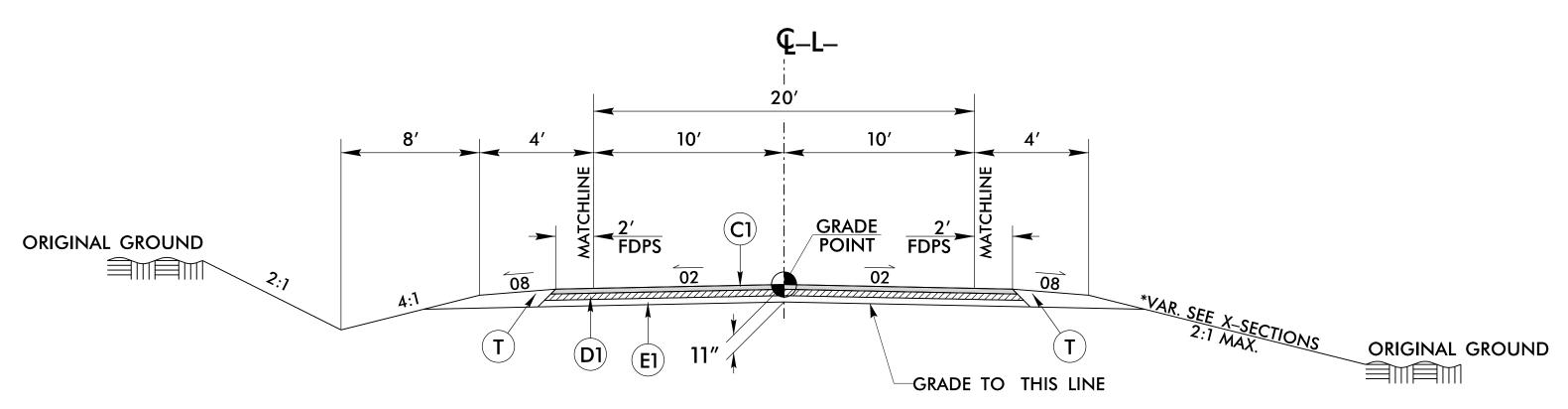
SHOULDER BERM GUTTER.

EARTH MATERIAL.

E1

WEDGING (SEE THIS SHEET FOR WEDGING DETAIL).

NOTE: PAVEMENT EDGE SLOPES ARE 1:1, UNLESS SHOWN OTHERWISE



TYPICAL SECTION NO. 1

-L- STA. 12 + 00.00 TO -L- STA. 14 + 18.75 (BEGIN BRIDGE) -L- STA. 15 + 86.25 (END BRIDGE) TO -L- STA. 17 + 50.00 ROADWAY DESIGN
ENGINEER

SEAL

O45230

SEAL

O45230

Docusigned by:

7/23/2018

Michael S. Burns, Jr.

D425C8CC008F437...

Firm Ucense No. C-1051

223 S. West St.

SUBJECT

PAVEMENT DESIGN
ENGINEER

SEAL

22896

Clark Morrison

BUATTODDTE004C4...

BUATTODDTE004C4...

STEWART

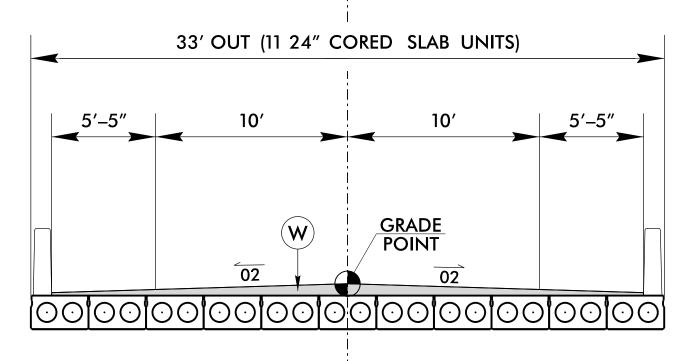
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SHEET NO.

PROJECT REFERENCE NO.

* 1.5:1 SLOPES FROM _L_ STA. 15 + 85 +/- TO 16 + 83 +/- LT.

Q _L_



TYPICAL SECTION NO. 2

-L- STA. 14+18.75 (BEGIN BRIDGE) TO -L- STA. 15+86.25 (END BRIDGE)

NOTE:

FOR ASPHALT DEPTHS, SEE STRUCTURE PLANS

STALL CLICOMINE

5'-5" EOL

2'-4" 3'-1" FDPS

R1 D1 E1

11"

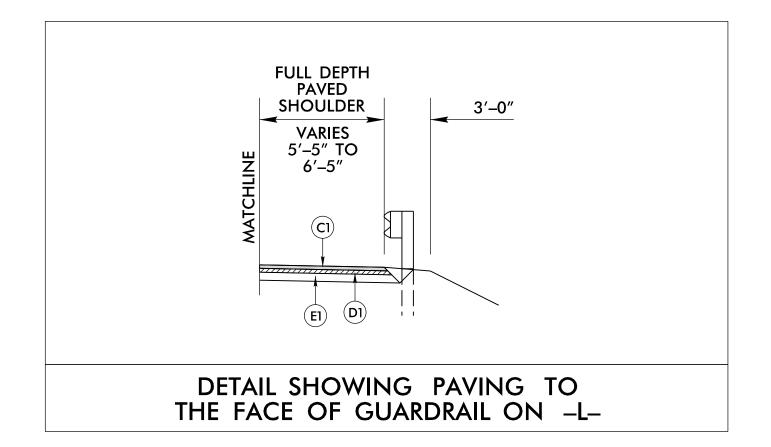
GRADE TO THIS LINE

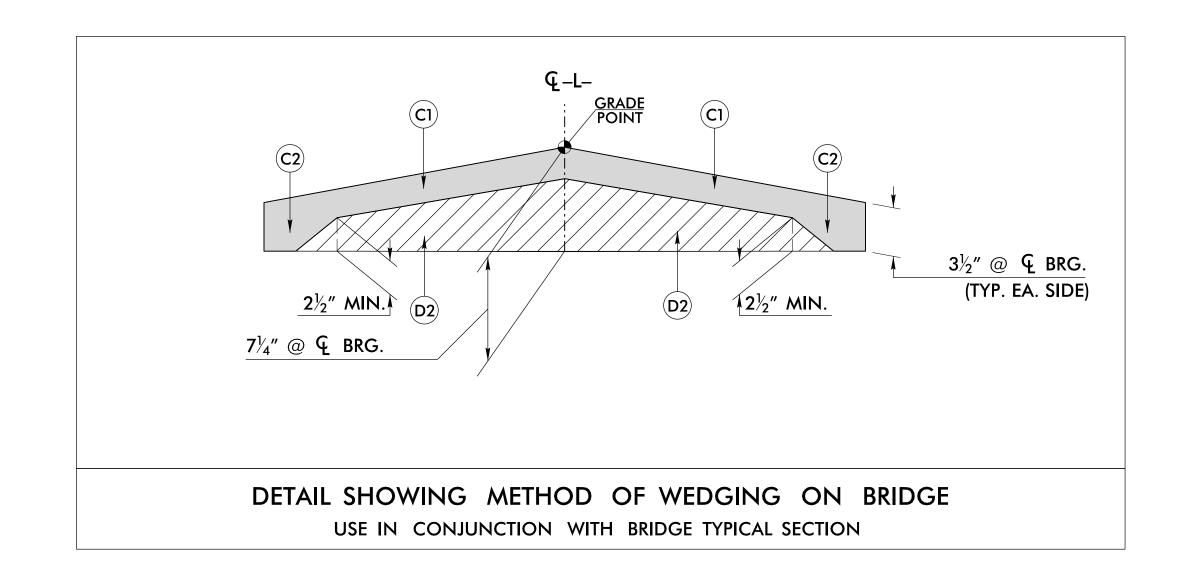
DETAIL SHOWING GUARDRAIL WITH SHOULDER BERM GUTTER

USE SHOULDER BERM GUTTER AT THE FOLLOWING LOCATIONS:

-L- STA. 15 + 97.13 (END APPROACH SLAB) TO -L- STA. 16 + 11.00 (LEFT)

-L- STA. 15 + 97.13 (END APPROACH SLAB) TO -L- STA. 16 + 11.00 (RIGHT)



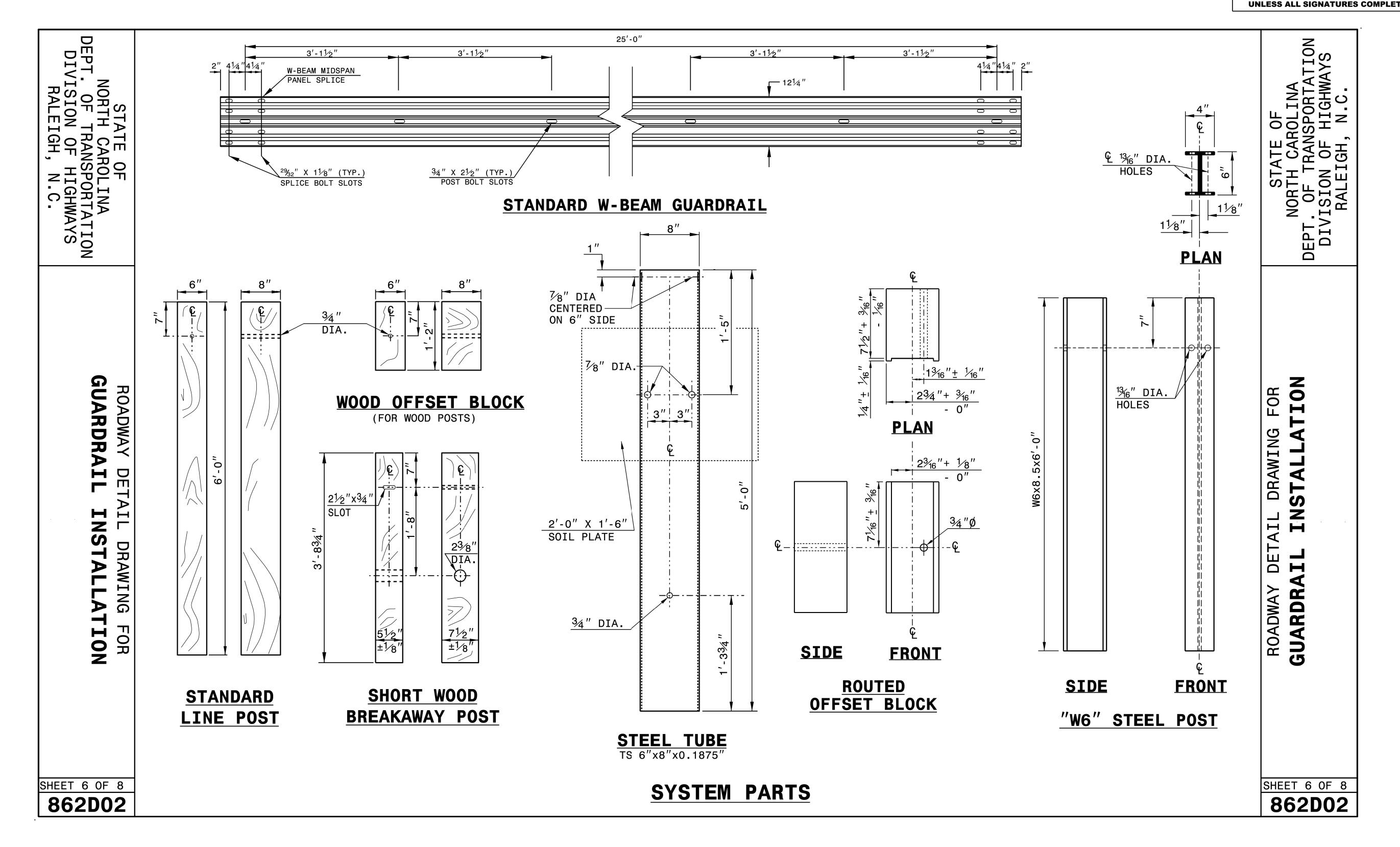


PROJECT REFERENCE NO. SHEET NO.

17BP.5.R.77

2C-1

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CONTRACTS STANDARDS AND DEVELOPMENT UNIT Office 919-707-6950 FAX 919-250-4119

SEE TITLE BLOCK

ORIGINAL BY: J.HOWERTON	DATE: <u>3-7-2018</u>
MODIFIED BY:	DATE:
CHECKED BY:	DATE:
FILE SPEC.:	

SHEET 2 OF 7
862D03 STATE OF NORTH CAROLINA DEPT. OF TRANSPORTATION SYAWBYS SYAWBY N.C. BALEIGH, N.C. STATE OF NORTH CAROLINA DEPT. OF TRANSPORTATION SYAMBYS SYAMAYS SYAMAY SYAMAYS SYAMAY SYAMAY SYAMAY SYAMAY SYAMAYS SYAMAY SYAMAY SYAMAY SYAMAY SYAMAY SYAMAY S 862D03 RAIL ON BRIDGE - SUB REGIONAL TIER FOR ATTACHMENT TO RAIL ON BRIDGE STRUCTURE ANCHOR UNIT, TYPE III GUARDRAIL ANCHOR UNIT, TYPE III FOR ATTACHMENT TO ROADWAY DETAIL DRAWING FOR ROADWAY DETAIL DRAWING FOR 10 III FOR ATTACHMENT REGIONAL TIER TYPE III ON BRIDGE EAK POINT EAK POINT UNIT, RAIL IL ANCHOR PAY LIMITS
UARDRAIL 'NESTED'
INSIDE ANOTHER) 4 5 GUARDRAI FOR ATTA GUARDRAIL ANCHOR RAIL ON B STATE OF
NORTH CAROLINA
DEPT. OF TRANSPORTATION
DIVISION OF HIGHWAYS 862D03 ROADWAY DETAIL DRAWING FOR SHEET 1 OF 7 862D03 ROADWAY DETAIL DRAWING FOR STATE OF
NORTH CAROLINA
DEPT. OF TRANSPORTATION
DIVISION OF HIGHWAYS STRUCTURE ANCHOR UNITS
GUARDRAIL ANCHOR UNIT, TYPE III FOR ATTACHMENT TO STRUCTURE ANCHOR UNITS
GUARDRAIL ANCHOR UNIT, TYPE III RAIL ON BRIDGE - SUB REGIONAL TIER FOR ATTACHMENT TO RAIL ON BRIDGE RALEIGH, N.C. RALEIGH, N.C.

PROJECT REFERENCE NO. SHEET NO. 17BP.5.R.77 2C-2

7/26/2018

SEAL

022966

OCCUSigned by:

Joel S. Howerton

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

CONTRACT STANDARDS AND DEVELOPMENT UNIT Office 919-707-6950 FAX 919-250-4119

SEE TITLE BLOCK

ORIGINAL BY: J HOWERTON	DATE: <u>06-22-12</u>
MODIFIED BY:	DATE:
CHECKED BY:	DATE:
FILE SPEC.:	

SUMMARY OF EARTHWORK

IN CUBIC YARDS

Station	Station	Uncl. Excav.	Embank. +%	Borrow	Waste
-L- Sta. 12+00.00	-L- Sta. 14+18.75	97	709	612	
-L- Sta. 15+86.25	-L- Sta. 17+50.00	143	269	126	
SUBTO	OTALS:	240	978	738	
PROJECT	Γ TOTALS	240	978	738	
EST. 5% REPLACE TO	PSOIL ON BORROW PIT			37	
GRAND '	TOTALS:	240		775	
SA	AY:	260		820	

UNDERCUT EXCAVATION = 400 CY (Contingency)
SELECT GRANULAR MATERIAL = 400 CY (Contingency)
GEOTEXTILE FOR SOIL STABILIZATION = 400 SY (Contingency)

(Total square yards of Geotextile for Soil Stabilization is only the contingent quantity and may only represent a portion of the geotextile quantity shown in the Item Sheets of the Proposal.)

Note: Approximate quantities only. Unclassified Excavation, Borrow Excavation, Fine Grading, Clearing and Grubbing, and Removal of Existing Pavement will be paid for at the contract lump sum price for Grading.

Note: Earthwork quantities are calculated by the Roadway Design Unit. These earthwork quantities are based on subsurface data provided by the Geotechnical Engineering Unit.

SHOULDER BERM GUTTER SUMMARY

IN LINEAR FEET

LINE	Station	Station	LENGTH
-L- (LT)	15+97.13	16+11.00	13.87
-L- (RT)	15+97.13	16+11.00	13.87
		TOTAL:	27.74
		SAY:	30

PAVEMENT REMOVAL SUMMARY

IN SQUARE YARDS

SURVEY LINE	Station	Station	LOCATION LT/RT/CL	ASPHALT REMOVAL	ASPHALT BREAKUP	CONCRETE REMOVAL	CONCRETE BREAKUP
-L-	12+00.00	14+28.59	CL	507.17			
-L-	15+66.05	17+50.00	CL	412.71			
		TOTAL:		919.88			
		SAY:		925			

"N" = DISTANCE FROM EDGE OF LANE TO FACE OF GUARDRAIL

TOTAL SHOULDER WIDTH = DISTANCE FROM EDGE OF TRAVEL LANE TO SHOULDER BREAK POINT.

FLARE LENGTH = DISTANCE FROM LAST SECTION OF PARALLEL GUARDRAIL TO END OF GUARDRAIL

W = TOTAL WIDTH OF FLARE FROM BEGINNING OF TAPER TO END OF GUARDRAIL

GUARDRAIL SUMMARY

G = GATING IMPACT ATTENUATOR TYPE 350 NG = NON-GATING IMPACT ATTENUATOR TYPE 350

Ī	SURVEY	BEG. STA.	END STA.	LOCATION		LENGTH		WARRANT	Γ POINT	"N" DIST.	TOTAL SHOUL	FLARE LI	ENGTH	V	V			ANCHORS			IMPACT ATTENUATOR TYPE 350	SINGLE FACED	REMOVE EXISTING	REMOVE & STOCKPILE	REMARKS
	LINE				STRAIGHT	SHOP CURVED	DOUBLE FACED	APPROACH END	TRAILING END	FROM E.O.L.	WIDTH	APPROACH END	TRAILING END	APPROACH END	TRAILING END	XI MOD XI	GREU TL-3	M-350 TYPE III CA	T-1 MOD	ВІС	G NG	CONCRETE BARRIER	GUARDRAIL	EXISTING GUARDRAIL	
	-L-	13+45.75	14+18.75	LT	75'				14+18.75	5'-5"	8'-5"		50'		1'		1	1							
	-L-	13+45.75	14+18.75	RT	75'			14+18.75		5'-5"	8'-5"	50'		1'			1	1							
	-L-	15+86.25	17+00.00	LT	113.75'			15+86.25		5'-5"	8'-5"	50'		1'			1	1							
	-L-	15+86.25	16+67.50	RT	81.25'				15+86.25	5'-5"	8'-5"		50'		1'		1	1							
С																									
9																									
38				SUBTOTAL:	345'												4	4							
Σ			LESS ANCHO	R DEDUCTIONS:																					
SU			TY	PE III (4@18.75)	75.00'																				
<u>></u>			GRE	EU, TL-3 (4@50')	200.00'																				
77_																									
307				TOTAL:	70'												4	4							
326 201				SAY:	87.5'												4	4							
∞/ <u>`</u> _0																									
20			ADDITION	AL GUARDRAIL P	OSTS= 5 EA																				
√ .S S:S																									

REVISIONS

COMPUTED BY:	EAB	DATE:	10/23/2017
CHECKED BY:	ECOLOGICAL ENGINEERING, LLP	DATE:	10/23/2017

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS

 PROJECT REFERENCE NO.
 SHEET NO.

 17BP.5.R.77
 3D-1

Note: Invert Elevations indicated are for Bid Purposes only and shall not be used for project construction stakeout. See "Standard Specifications For Roads and Structures, Section 300-5".

LIST OF PIPES		TEAD DIDEC 10	
I INTERPRES	. PUNITO ALUAN.	IPUK PIPPA 4X	·

STATION	STRUCTURE NO.	TOP ELEVATION INVERT ELEVATION	INVERT ELEVATION SLOPE CRITICAL	12" 15"		DRAINAGE PIP SSP, CAAP, HDPE, 30" 36" 42"		CAAP	C.S. PIPE	42" 48" 12	R.C. PIPE CLASS III " 15" 18" 24" 30" 36" 42" 48"	12" 15"	R.C. PIPE CLASS IV 18" 24" 30" 36" 42" 48"	(A)	FS, CONTRACTOR DESIGN	rs, contractor design	-	CO. YARDS CO. YARDS	'A' + (1.3 X COL.'B')	FRAME, GRATES, AND HOOD STANDARD 840.03	SECTION 1D. 840.15	D GRATES STD. 840.16 340.17 OR 840.26 340.18 OR 840.27 340.19 OR 840.28	NO GRATES STD. 840.22	1TH TWO GRATES STD. 840.24	AME W/ 2 GRATES STD. 840.29	OWS NO. & SIZE	"B" C.Y. STD. 840.72	Ť.	C.B. N.D.I. D.I. G.D.I. G.D.I.(N.S.) J.B. M.H. T.B.D.I.	CATCH BASIN NARROW DROP INLET DROP INLET GRATED DROP INLET (NARROW SLOT) JUNCTION BOX MANHOLE TRAFFIC BEARING
THICKNESS OR GAUGE	FROM					DO NOT USE PY	DO NOT USE R	DO NOT USE HD	.064 .064 .064 .079	.109				*" R.C. PIPE (CLASS	*" RC PIPE CULVER"	*" RC PIPE CULVER'	S" SIDE DRAIN PIPE	C.S.P. THRU 10.0'	0.0' AND ABOVE	TYPE OF GRATE	ATCH BASIN .I. STD. 840.14 OR ST	D.I. FRAME WITH TWC G.D.I. TYPE "A" STD. 8 G.D.I. TYPE "B" STD. 8 G.D.I. TYPE "D" STD. 8	FRAME V	D.I. (N.S.) FRAME W D.I. (N.S.) FRAME W B. STD. 840.31 OR 8	.D.I. (N.S. FLAT) FR, B.D.I. STD. 840.35	DRAINAGE PIPE ELE	ONC. COLLARS CL.	PE REMOVAL LIN. F	T.B.J.B.	DROP INLET TRAFFIC BEARING JUNCTION BOX EMARKS
16+06 -L- L	Г 0401	170.98 168.23												*	*	* +	- =	1 1	- 3				9 9 9	ອ່ ຜ່ - 	1 1		0 0	<u> </u>	.,	
	0401 0402		8.13									28																		
16+06 -L- R	T 0402	170.98 168.13																1							1 1					
	0402 0403	168.13 16	4.00	20		X	Х	X																		2@15'				
SHEET TOTALS				20	-					++		28					+	2			+ + +				2 2	2@15'				

REVISIO

PROJECT REFERENCE NO.	SHEET NO.
17BP.5.R.77	3G-I

SUMMARY OF ROCK PLATING

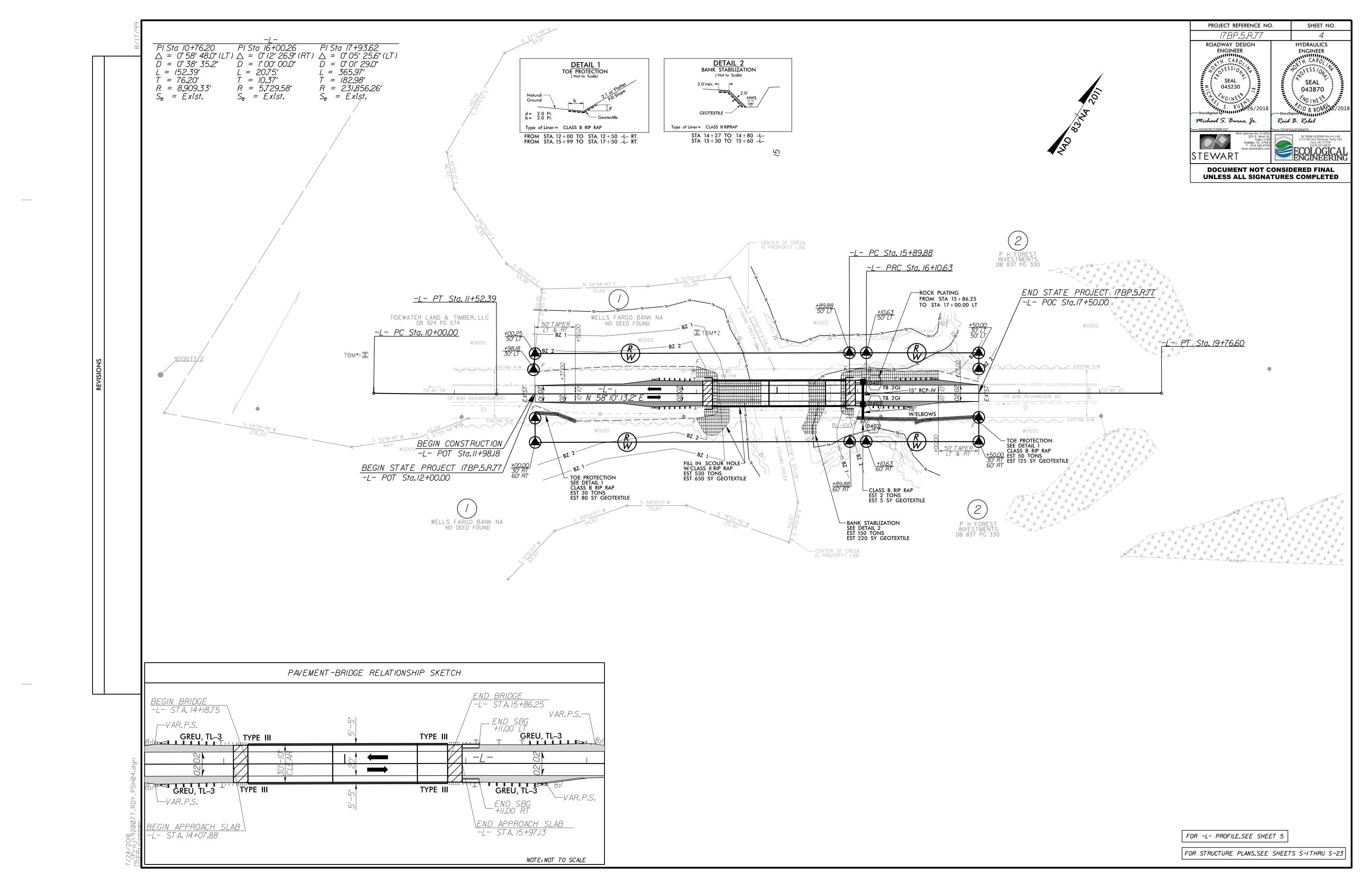
LINE	Beginning Slope	Approx. Station	Ending Slope	Approx. Station	Location LT/RT	Rock Plating Detail No. 1/2/3/4	Riprap Class* I/II/B	SY
-L-	1.5:1	15+86.25	1.5:1	17+00.00	LT	1	II	132
							TOTAL SY:	132

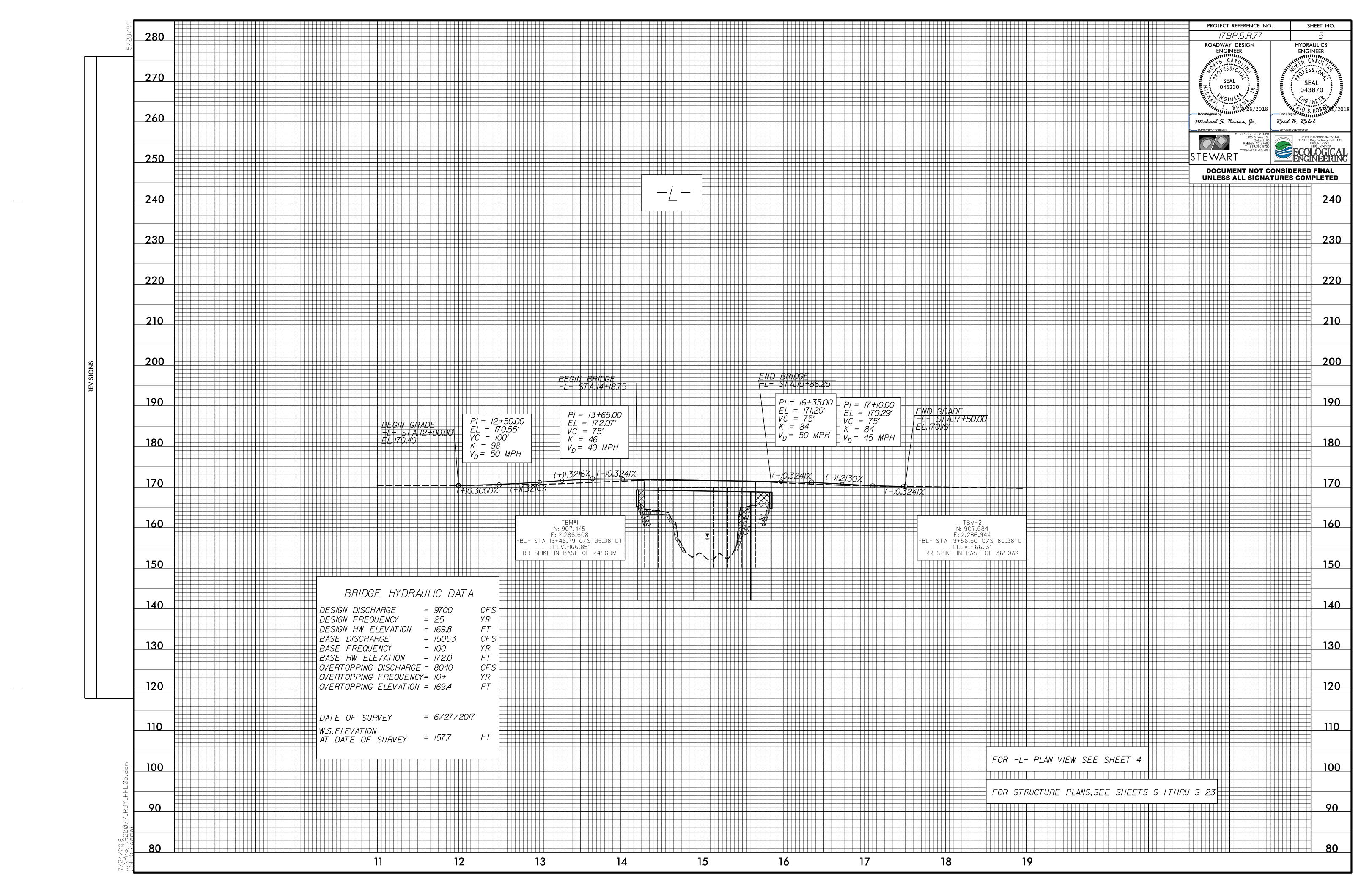
^{*}Use Class I, II or B riprap if riprap class is not shown for rock plating location.

SUMMARY OF AGGREGATE SUBGRADE/STABILIZATION

LINE	Station	Station	Aggregate Type ASU/AST	Aggregate Thickness INCHES	Shallow Undercut CY	Class IV Subgrade Stabilization TONS	Geotextile for Soil Stabilization SY	Stabilizer Aggregate TONS	Class IV Aggregate Stabilization TONS
(CONTINGENC	Y	ASU		100	200	300		
			TOTAL (CY/TONS/SY:	100	200	300*		

ASU = Aggregate Subgrade, AST = Aggregate Stabilization
*Total square yards of Geotextile for Soil Stabilization is only the estimated quantity for ASU/AST and may only represent a portion of the geotextile quantity shown in the Item Sheets of the Proposal.

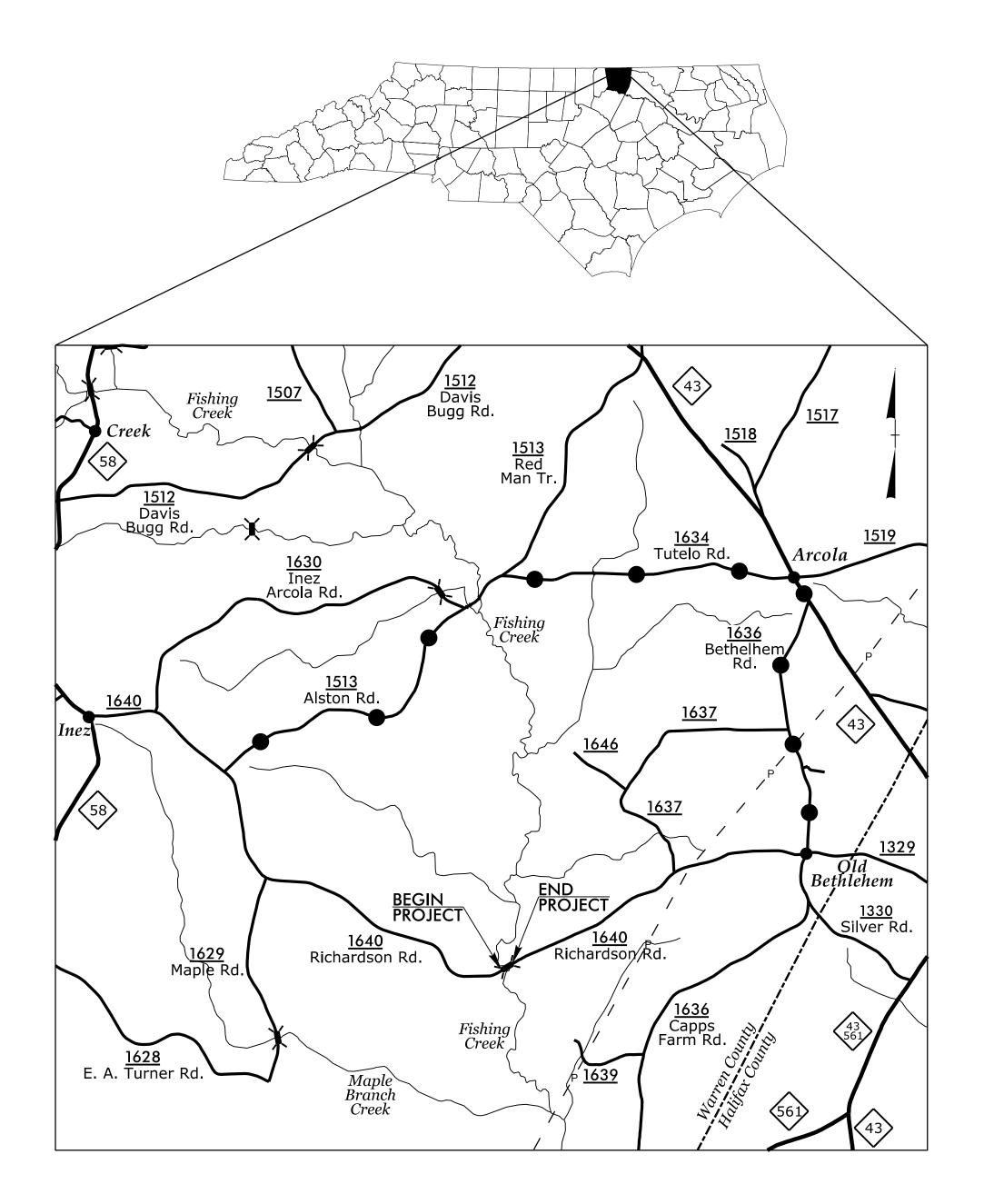




STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

TRANSPORTATION MANAGEMENT PLAN

WARREN COUNTY



INDEX OF SHEETS

SHEET NO. <u>TITLE</u>

TITLE SHEET, VICINITY MAP, AND INDEX OF SHEETS TMP - 1

SHEET NO.

TMP-1

LIST OF APPLICABLE ROADWAY STANDARD DRAWINGS, AND LEGEND

TRANSPORTATION OPERATIONS PLAN: (MANAGEMENT STRATEGIES, GENERAL NOTES, LOCAL NOTES, AND PHASING) TMP-1B

SPECIAL SIGN DESIGN TMP-2

OFF-SITE DETOUR TMP-3

TMP-1A

DOCUMENT NOT CONSIDERED FINAL **UNLESS ALL SIGNATURES COMPLETED**

APPROVED: Michael S. Burns, Jr. **DATE:** 7/26/2018 SEAL





ANDY YOUNG, PE PROJECT ENGINEER

MICHAEL BURNS, PE PROJECT DESIGN ENGINEER



ROADWAY STANDARD DRAWINGS

THE FOLLOWING ROADWAY STANDARDS AS SHOWN IN "ROADWAY STANDARD DRAWINGS" -N.C. DEPARTMENT OF TRANSPORTATION - RALEIGH, N.C., DATED JANUARY 2018 ARE APPLICABLE TO THIS PROJECT AND BY REFERENCE HEREBY ARE CONSIDERED A PART OF THESE PLANS:

STD. NO.

TITLE

1101 02	TEMPORARY ROAD CLOSURES
1101.03	TEMPURANT NUAD CLUSURES
1101.11	TRAFFIC CONTROL DESIGN TABLES
1110.01	STATIONARY WORK ZONE SIGNS
1130.01	DRUM
1145.01	BARRICADES

LEGEND

GENERAL

DIRECTION OF TRAFFIC FLOW

DIRECTION OF PEDESTRIAN TRAFFIC FLOW

----- EXIST. PVMT.

NORTH ARROW

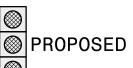
PROPOSED PVMT.

TEMP. SHORING (LOCATION PURPOSES ONLY)

WORK AREA

REMOVAL

SIGNALS





PAVEMENT MARKINGS

——EXISTING LINES ——TEMPORARY LINES

TRAFFIC CONTROL DEVICES

BARRICADE (TYPE III)

DRUM SKINNY DRUM STUBULAR MARKER

TEMPORARY CRASH CUSHION FLASHING ARROW BOARD

FLAGGER

LAW ENFORCEMENT

TRUCK MOUNTED ATTENUATOR (TMA)

CHANGEABLE MESSAGE SIGN

TEMPORARY SIGNING

PORTABLE SIGN

STATIONARY SIGN

STATIONARY OR PORTABLE SIGN

PAVEMENT MARKERS

CRYSTAL/CRYSTAL

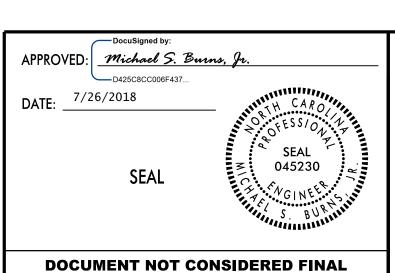
CRYSTAL/RED

◆ YELLOW/YELLOW

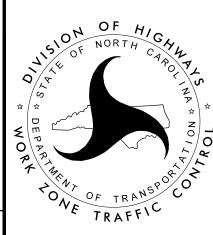
PAVEMENT MARKING SYMBOLS

PAVEMENT MARKING SYMBOLS

Firm License No. C-1051 223 S. West St, Suite 1100 Raleigh, NC 27603 T 919.380.8750 www.stewartinc.com STEWART



UNLESS ALL SIGNATURES COMPLETED



ROADWAY STANDARD DRAWINGS & LEGEND

PROJ. REFERENCE NO. SHEET NO. 17BP.5.R.77 TMP-1B

MANAGEMENT STRATEGIES

DURING CONSTRUCTION OF PROPOSED STRUCTURE BRIDGE No. 77 OVER FISHING CREEK, SR 1640 (RICHARDSON RD.) WILL BE CLOSED TO THROUGH TRAFFIC. THROUGH TRAFFIC ON SR 1640 (RICHARDSON RD.) WILL BE MAINTAINED USING AN OFF-SITE DETOUR.

THE OFF-SITE DETOUR WILL INCLUDE SR 1513, SR 1634, NC 43, AND SR 1133 (SEE SHEET TMP-3).

GENERAL NOTES

CHANGES MAY BE REQUIRED WHEN PHYSICAL DIMENSIONS IN THE DETAIL DRAWINGS, STANDARD DETAILS, AND ROADWAY DETAILS ARE NOT ATTAINABLE TO MEET FIELD CONDITIONS OR RESULT IN DUPLICATE OR UNDESIRED OVERLAPPING OF DEVICES. MODIFICATION MAY INCLUDE: MOVING, SUPPLEMENTING, COVERING, OR REMOVAL OF DEVICES AS DIRECTED BY THE ENGINEER.

THE FOLLOWING GENERAL NOTES APPLY AT ALL TIMES FOR THE DURATION OF THE CONSTRUCTION PROJECT EXCEPT WHEN OTHERWISE NOTED IN THE PLAN OR DIRECTED BY THE ENGINEER.

SIGNING

A) PROVIDE SIGNING AND DEVICES
REQUIRED TO CLOSE THE ROAD ACCORDING TO THE ROADWAY STANDARD
DRAWINGS AND TRAFFIC CONTROL PLANS.

PROVIDE SIGNING REQUIRED FOR THE OFF-SITE DETOUR ROUTE AS SHOWN IN THE TRAFFIC CONTROL PLANS.

B) COVER OR REMOVE ALL SIGNS AND DEVICES REQUIRED TO CLOSE THE ROAD WHEN ROAD CLOSURE IS NOT IN OPERATION.

COVER OR REMOVE ALL SIGNS REQUIRED FOR THE OFF-SITE DETOUR WHEN THE DETOUR IS NOT IN OPERATION.

C) ENSURE ALL NECESSARY SIGNING IS IN PLACE PRIOR TO ALTERING ANY TRAFFIC PATTERN.

LOCAL NOTES

- 1. NOTIFY THE ENGINEER AT LEAST 30 DAYS PRIOR TO ANY TRAFFIC PATTERN ALTERATION.
- 2. NOTIFY THE WARREN COUNTY SCHOOLS TRANSPORTATION DIRECTOR OF THE BRIDGE REMOVAL 30 DAYS PRIOR TO ROAD CLOSURE.
- 3. NOTIFY THE WARREN COUNTY EMERGENCY MANAGEMENT SERVICES DIRECTOR OF BRIDGE REMOVAL 30 DAYS PRIOR TO ROAD CLOSURE.

PHASING

STEP 1:

PROVIDE AND MAINTAIN CHANGEABLE MESSAGE SIGNS AT EACH END OF SR 1640 (RICHARDSON RD.) FOR FOURTEEN (14) CALENDAR DAYS PRIOR TO ROAD CLOSURE, AS SHOWN ON SHEET TMP-3.

<u>STEP 2:</u>

USING RSD 1101.03, SHEET 1 OF 9, SHEETS TMP-2 AND TMP-3, INSTALL ROAD CLOSURE AND DETOUR SIGNS, PLACE TYPE III BARRICADES TO CLOSE SR 1640 (RICHARDSON RD.) TO THROUGH TRAFFIC, AND DETOUR TRAFFIC OFF-SITE. REMOVE CHANGEABLE MESSAGE SIGNS ONCE DETOUR IS IN PLACE.

STEP 3:

REMOVE THE EXISTING STRUCTURES.

STEP 4:

CONSTRUCT THE PROPOSED STRUCTURES AND ROADWAY.

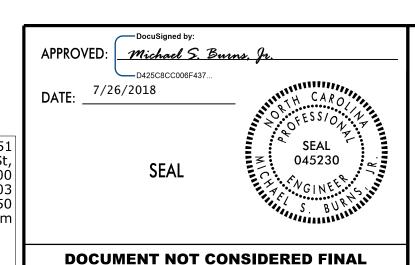
STEP 5:

PLACE FINAL PAVEMENT MARKINGS ACCORDING TO THE PAVEMENT MARKING PLANS.

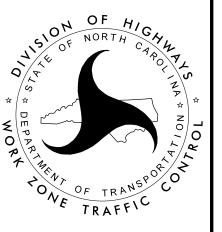
<u>STEP 6:</u>

OPEN SR 1640 (RICHARDSON RD.) TO TRAFFIC AND REMOVE ALL WORK ZONE TRAFFIC CONTROL DEVICES.





UNLESS ALL SIGNATURES COMPLETED



TRANSPORTATION
OPERATIONS
PLAN

.\TCP\920077_TC_TMP_OIB.dgn SER:iroemer

PROJ. REFERENCE NO. SHEET NO. 17BP.5.R.77 TMP-2

SIGN NUMBER: RICHARDSON RD BACKG COLOR: Fluorescent Orange TYPE: STATIONARY COPY COLOR: Black QUANTITY: SEE PLANS SYMBOL WID HT X Υ SIGN WIDTH: 3'-6" **HEIGHT:** 1'-6" TOTAL AREA: 10.0 Sq.Ft. **BORDER TYPE: INSET RECESS: 0.38**" WIDTH: 0.63" **RADII:** 1.5" MAT'L: 0.080" (2.0 mm) ALUMINUM NO. Z BARS:

USE NOTES: 1,2

- Legend and border shall be direct applied black non-reflective sheeting.
- 2.Background shall be NC GRADE B fluorescent orange retroreflective sheeting.

DESIGN BY: Michael Burns, PE
PROJECT ID: 17BP.5.R.77

CHECKED BY: Andy Young, PE
LOCATION: Franklin County

Feb 07, 2018 DIV: 5



R=1.5" TH=0.63"

IN=0.38" Panel Style: Traffic Control.ssi

M.U.T.C.D.: 2009 Edition

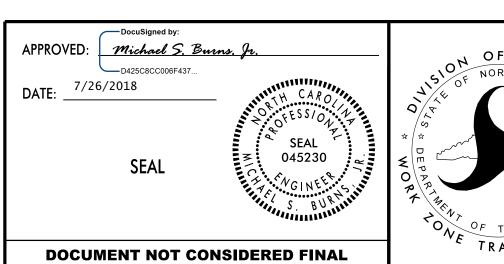
Spacing Factor is 1 unless specified otherwise

LETTER POSITIONS

LENGTH:

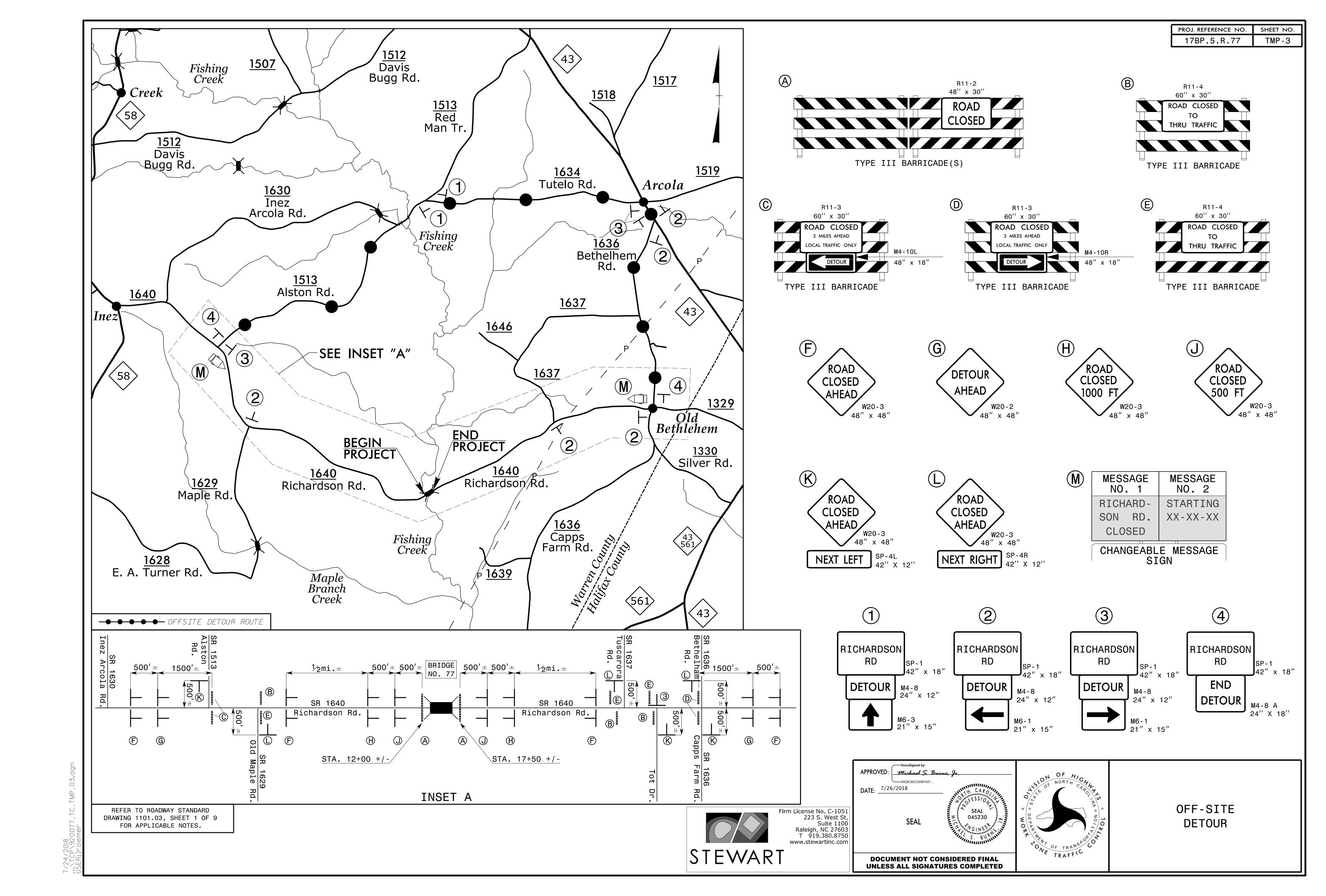
Letter locations are panel edge to lower left corner	Series/Siz Text Lengt
RICHARDSON	D 2000
4.8 8.2 9.8 13.4 16.7 20.7 24.1 27.4 30.7 34.4	32.3
RD	D 2000
17.9 21.3	6.1





UNLESS ALL SIGNATURES COMPLETED





TIP: 17BP.5.R.77

CONTRACT

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

PAVEMENT MARKING PLAN WARREN COUNTY

LOCATION: BRIDGE NO. 77 OVER FISHING CREEK ON SR 1640 (RICHARDSON RD.)

 TIP NO.
 SHEET NO.

 17BP.5.R.77
 PMP-1

APPROVED: Michael S. Burns, Jr.

SFAI ·

DATE: 7/26/2018



DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

ROADWAY STANDARD DRAWING

THE FOLLOWING ROADWAY STANDARDS AS APPEAR IN "ROADWAY STANDARD DRAWINGS" - PROJECT SERVICES UNIT - N.C. DEPARTMENT OF TRANSPORTATION - RALEIGH, N.C., DATED JANUARY 2018 ARE APPLICABLE TO THIS PROJECT AND BY REFERENCE HEREBY ARE CONSIDERED A PART OF THESE PLANS:

STD. NO.	TITLE
1005 01	DAVEMENT MADIZINGS I INC TYPES AND OFFSETS
1205.01	PAVEMENT MARKINGS - LINE TYPES AND OFFSETS
1205.02	PAVEMENT MARKINGS - TWO-LANE AND MULTILANE ROADWAYS
1205.12	PAVEMENT MARKINGS - BRIDGES
1261.01	GUARDRAIL AND BARRIER DELINEATORS - INSTALLATION SPACING
1261.02	GUARDRAIL AND BARRIER DELINEATORS - TYPES AND MOUNTING
1262.01	GUARDRAIL END DELINEATION

PAVEMENT MARKING SCHEDULE

SYMBOL	DESCRIPTION
PA	PAINT WHITE EDGELINE (4") X2
ΡI	PAINT YELLOW DOUBLE CENTER (4") X2



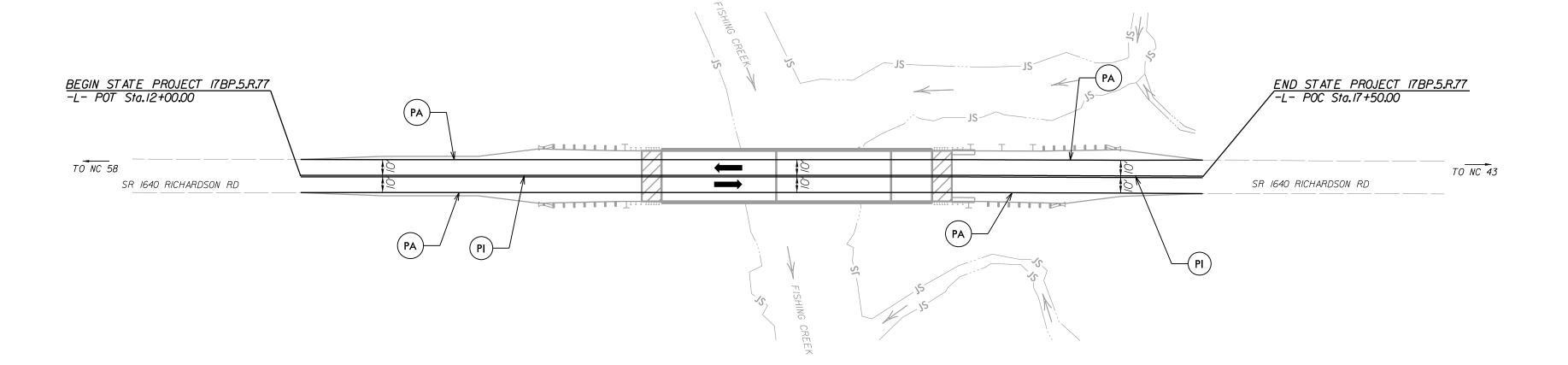
GENERAL NOTES

THE FOLLOWING GENERAL NOTES APPLY AT ALL TIMES FOR THE DURATION OF THE CONSTRUCTION PROJECT, EXCEPT WHEN OTHERWISE NOTED IN THE PLAN, OR DIRECTED BY THE ENGINEER.

A) INSTALL PAVEMENT MARKINGS AND PAVEMENT MARKERS ON THE FINAL SURFACE AS FOLLOWS:

ROAD NAME MARKING MARKER
RICHARDSON RD PAINT NONE

- B) PLACE TWO APPLICATIONS OF PAINT PAVEMENT MARKINGS ON THE FINAL WEARING SURFACE. PLACE THE SECOND APPLICATION OF PAINT UPON SUFFICIENT DRYING TIME OF THE FIRST.
- C) TIE PROPOSED PAVEMENT MARKING LINES TO EXISTING PAVEMENT MARKING LINES.
- D) REMOVE/REPLACE ANY CONFLICTING/DAMAGED PAVEMENT MARKINGS.
- E) PASSING ZONES WILL BE DETERMINED IN THE FIELD AND MUST BE APPROVED BY THE ENGINEER.



PLAN PREPARED BY: STEWART

ANDY YOUNG, PE

PROJECT ENGINEER

MICHAEL BURNS, PE

PROJECT DESIGN ENGINEER



1513 🎺 **PROJECT** VICINITY MAP OFF-SITE DETOUR — See Sheet 1A For Index of Sheets See Sheet 1B For Conventional Symbols

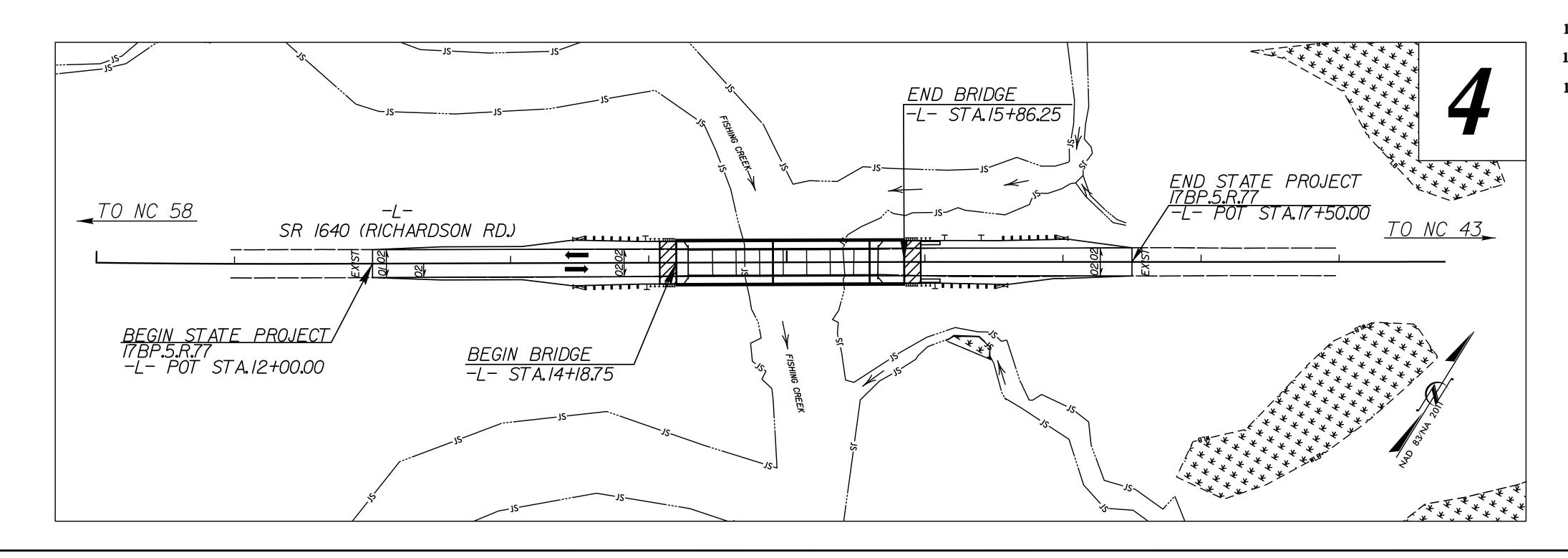
STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

PLAN FOR PROPOSED HIGHWAY EROSION CONTROL

WARREN COUNTY

LOCATION: BRIDGE NO. 77 OVER FISHING CREEK ON SR 1640 (RICHARDSON RD.)

TYPE OF WORK: GRADING, DRAINAGE, PAVING, AND STRUCTURE



NC FIRM LICENSE No: F-1148

STATE	STATE PROJECT REFERENCE NO.		SHEET NO.	TOTAL SHEETS
N.C.	12	7BP.5.R.77	EC-1	
STATE P	ROJ. NO.	F. A. PROJ. NO.	DESCRIPTI	ON
17BP.5	.R.77	N/A	PE	
				·

EROSION AND SEDIMENT CONTROL MEASURES

Séd. #	<u>Description</u>	Symbol
1630.03	Temporary Silt Ditch	
1630.05	Temporary Diversion	— то —
1605.01	Temporary Silt Fence	
1606.01	Special Sediment Control Fence	
1622.01	Temporary Berms and Slope Drains	T←←
1630.02	Silt Basin Type B	. ////
1633.01	Temporary Rock Silt Check Type-A	·····
	Temporary Rock Silt Check Type A w Matting and Polyacrylamide (PAM)	
1633.02	Temporary Rock Silt Check Type-B	
	Wattle / Coir Fiber Wattle	
	Wattle / Coir Fiber Wattle with Polyacrylamide (PAM)	(<u>)</u>
1634.01	Temporary Rock Sediment Dam Type-A	<u></u>
1634.02	Temporary Rock Sediment Dam Type-B	
1635.01	Rock Pipe Inlet Sediment Trap Type-A	······································
1635.02	Rock Pipe Inlet Sediment Trap Type-B.	🔰 🐭
1630.04	Stilling Basin	7000
1630.06	Special Stilling Basin	
	Rock Inlet Sediment Trap:	
1632.01	Туре А	A 🔲
1632.02	Туре В	В
1632.03	Туре С	C
	Skimmer Basin	
	Tiered Skimmer Basin	
	Infiltration Basin	
	THIS PROJECT	CONTAINS

THIS PROJECT HAS BEEN DESIGNED TO SENSITIVE WATERSHED STANDARDS.

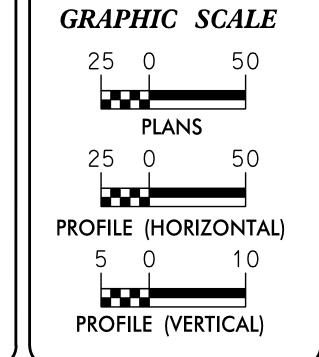
EROSION CONTROL PLANS

FOR CLEARING AND GRUBBING PHASE OF

CONSTRUCTION.

ENVIRONMENTALLY SENSITIVE AREA(S) EXIST ON THIS PROJECT

Refer To E. C. Special Provisions for Special Considerations.



THESE EROSION AND SEDIMENT CONTROL PLANS COMPLY WITH THE REGULATIONS SET FORTH BY THE NCG-010000 GENERAL CONSTRUCTION PERMIT EFFECTIVE AUGUST 1, 2016 AND ISSUED BY THE NORTH CAROLINA DEPARTMENT OF ENVIRONMENT AND NATURAL RESOURCES DIVISION OF WATER RESOURCES.

1151 SE Cary Parkway Suite 101 Cary, NC 27518 (919) 557-0929 Designed by: *3409* REID ROBOL, PE *NAME* LEVEL III CERTIFICATION NO.

Prepared in the Office of:

Reviewed in the Office of:

ROADSIDE ENVIRONMENTAL UNIT

1 South Wilmington St. Raleigh, NC 27611

2018 STANDARD SPECIFICATIONS

Reviewed by:

DONALD PEARSON

Roadway Standard Drawings

The following roadway english standards as appear in "Roadway Standard Drawings" – Roadway Design Unit – N. C. Department of Transportation – Raleigh, N. C., dated January 2018 and the latest revison thereto are applicable to this project and by reference hereby are considered a part of

1604.01 Railroad Erosion Control Detail 1605.01 Temporary Silt Fence 1606.01 Special Sediment Control Fence 1607.01 Gravel Construction Entrance 1622.01 Temporary Berms and Slope Drains

1630.01 Riser Basin 1630.02 Silt Basin Type B 1630.03 Temporary Silt Ditch

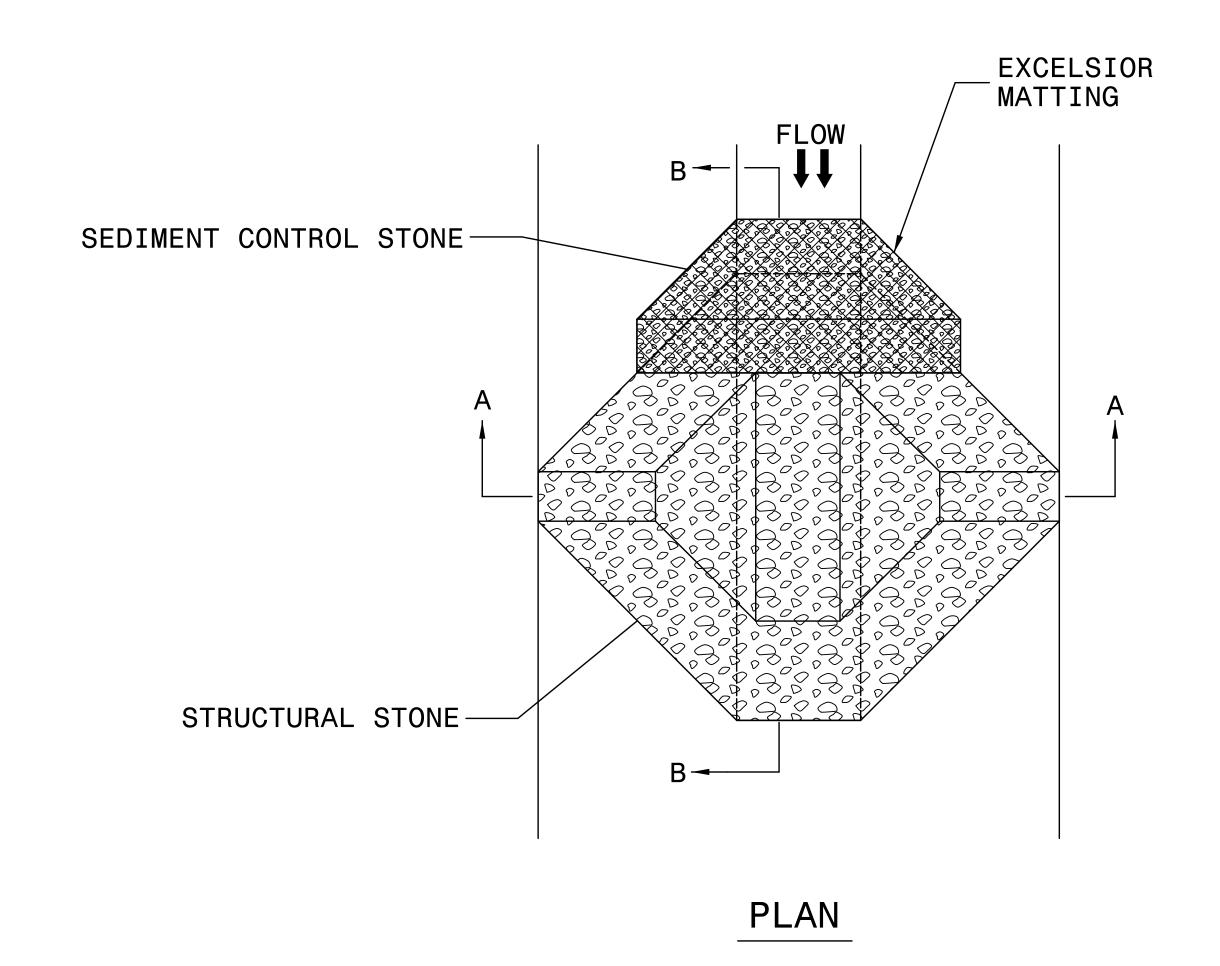
1630.04 Stilling Basin 1630.05 Temporary Diversion 1630.06 Special Stilling Basin 1631.01 Matting Installation 1632.01 Rock Inlet Sediment Trap Type A 1632.02 Rock Inlet Sediment Trap Type B 1632.03 Rock Inlet Sediment Trap Type C 1633.01 Temporary Rock Silt Check Type A 1633.02 Temporary Rock Silt Check Type B

1645.01 Temporary Stream Crossing

1634.01 Temporary Rock Sediment Dam Type A
1634.02 Temporary Rock Sediment Dam Type B
1635.01 Rock Pipe Inlet Sediment Trap Type A
1635.02 Rock Pipe Inlet Sediment Trap Type B
1640.01 Coir Fiber Baffle

TEMPORARY ROCK SILT CHECK TYPE 'A' WITH EXCELSIOR MATTING AND POLYACRYLAMIDE (PAM)

PROJECT REFERENCE NO	. SHEET NO.
17BP .5. R.77	EC-02
R/W SHEET N	O.
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER



See Inset A 2/3 CHANNEL WIDTH 1' MIN EXCELSIOR MATTING SECTION A-A

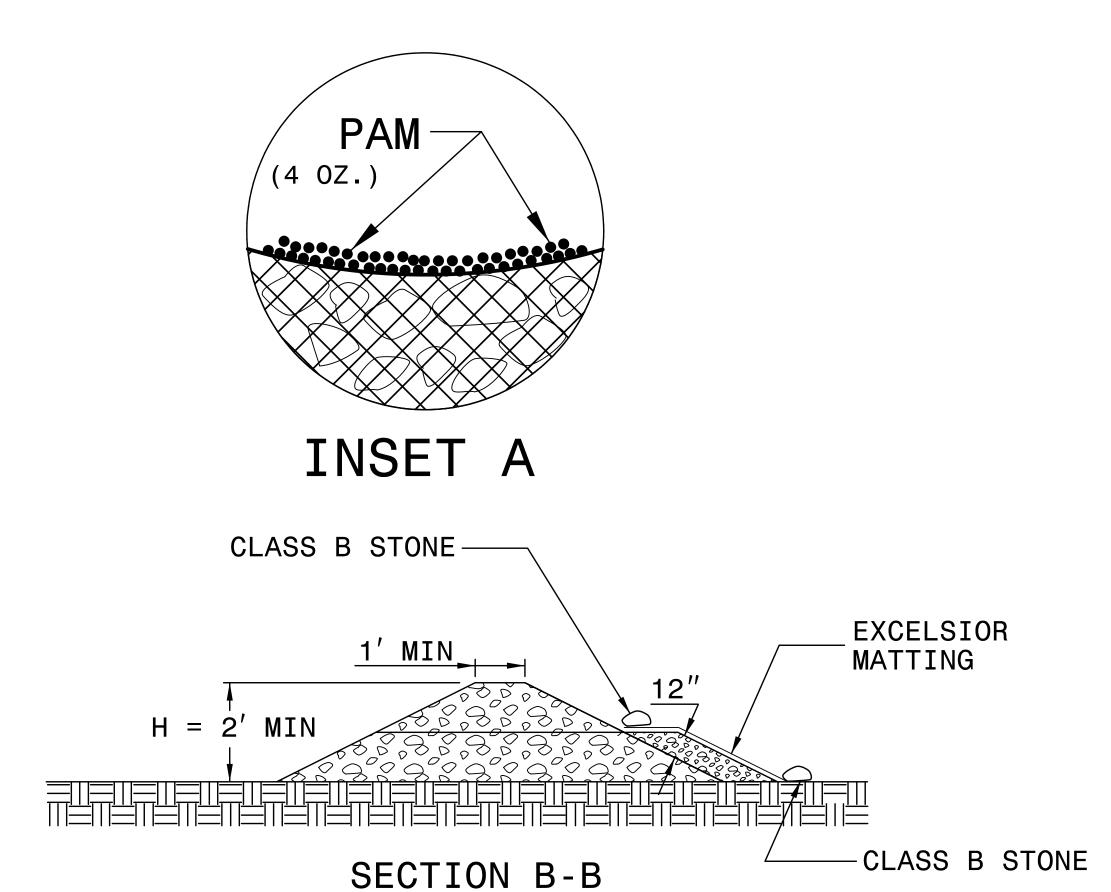
NOTES:

INSTALL TEMPORARY ROCK SILT CHECK TYPE A IN ACCORDANCE WITH ROADWAY STANDARD DRAWING NO. 1633.01.

USE EXCELSIOR FOR MATTING MATERIAL AND ANCHOR MATTING SECTION AT TOP AND BOTTOM WITH CLASS B STONE.

PRIOR TO POLYACRYLAMIDE (PAM) APPLICATION, OBTAIN A SOIL SAMPLE FROM PROJECT LOCATION, AND FROM OFFSITE MATERIAL, AND ANALYZE FOR APPROPRIATE PAM FLOCCULANT TO BE APPLIED TO EACH ROCK SILT CHECK.

INITIALLY APPLY 4 OUNCES OF POLYACRYLAMIDE (PAM) TO TOP OF MATTING SECTION AND AFTER EVERY RAINFALL EVENT THAT EQUALS OR EXCEEDS 0.50 INCHES.



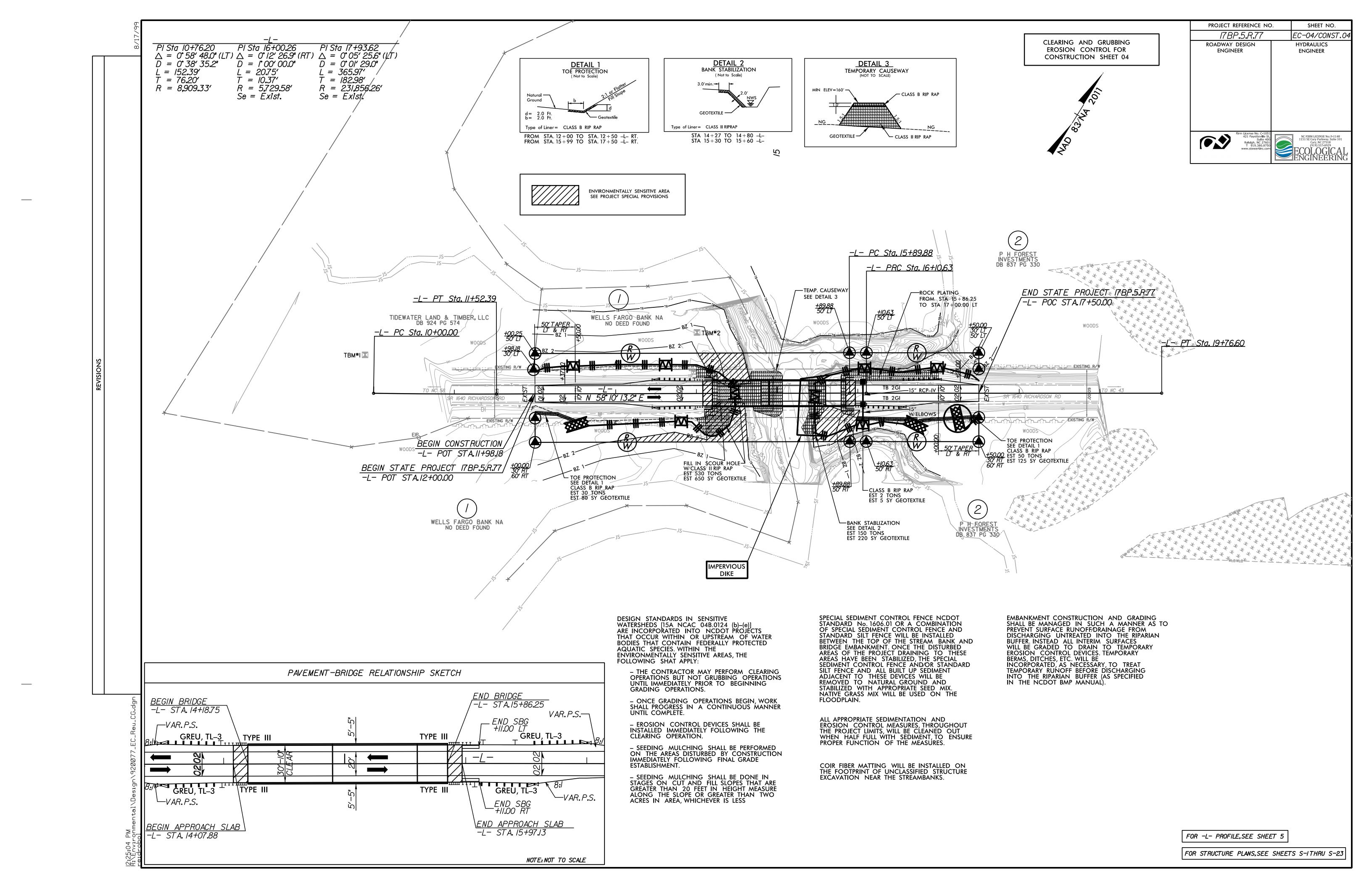
NOT TO SCALE

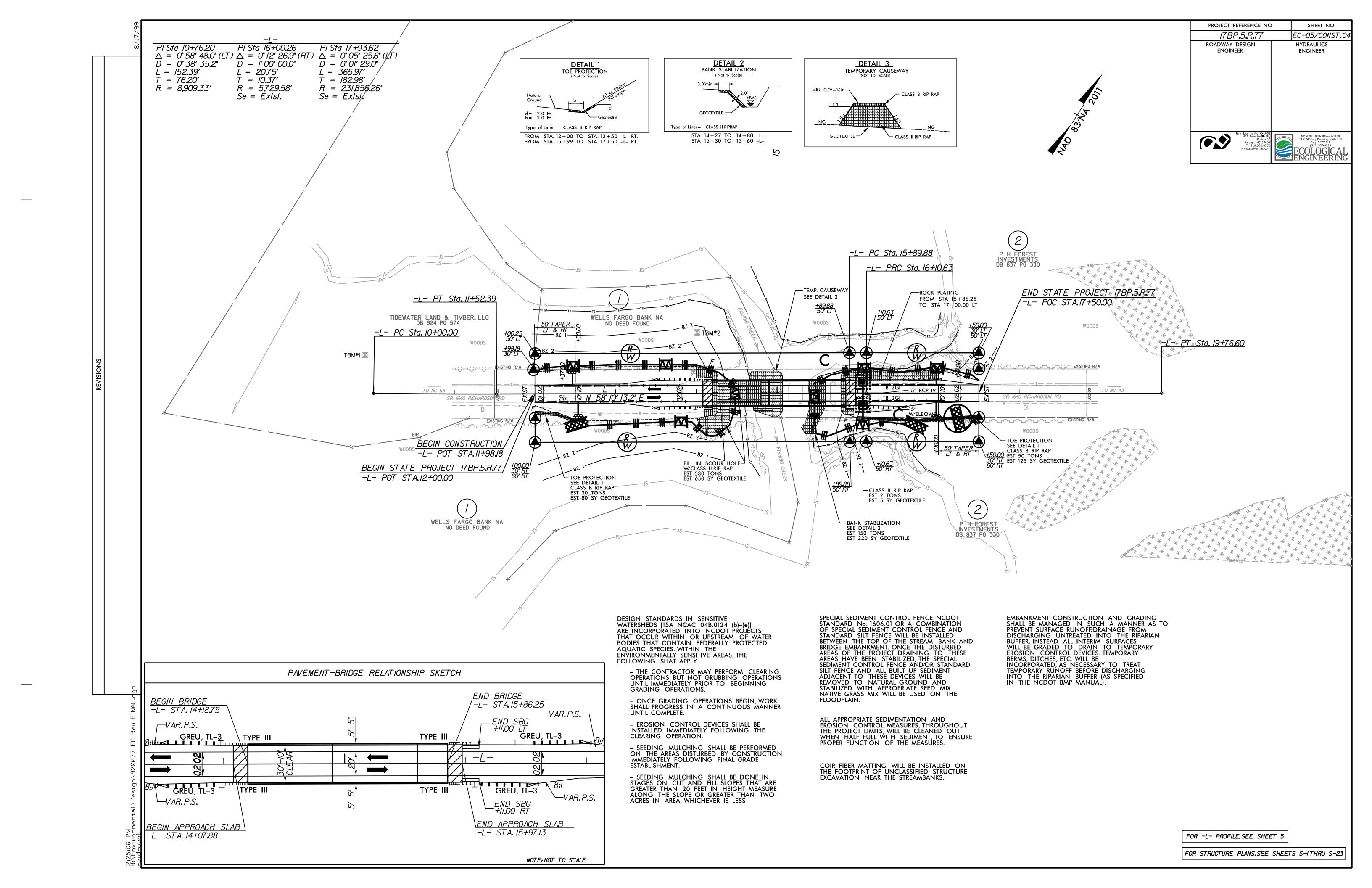
OJECT REFERENCE NO. SHEET NO. *EC-03*

DIVISION OF HIGHWAYS STATE OF NORTH CAROLINA

SOIL STABILIZATION TIMEFRAMES

SITE DESCRIPTION	STABILIZATION TIME	TIMEFRAME EXCEPTIONS
PERIMETER DIKES, SWALES, DITCHES AND SLOPES	7 DAYS	NONE
HIGH QUALITY WATER (HQW) ZONES	7 DAYS	NONE
SLOPES STEEPER THAN 3:1	7 DAYS	IF SLOPES ARE 10'OR LESS IN LENGTH AND ARE NOT STEEPER THAN 2:1,14 DAYS ARE ALLOWED.
SLOPES 3:1 OR FLATTER	7 DAYS	7 DAYS FOR SLOPES GREATER THAN 50'IN LENGTH.
ALL OTHER AREAS WITH SLOPES FLATTER THAN 4:1	7 DAYS	NONE, EXCEPT FOR PERIMETERS AND HQW ZONES.



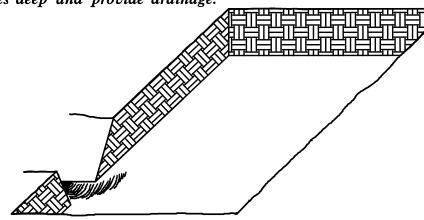


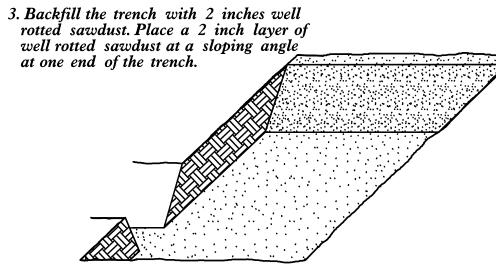
PLANTING DETAILS

SEEDLING / LINER BAREROOT PLANTING DETAIL

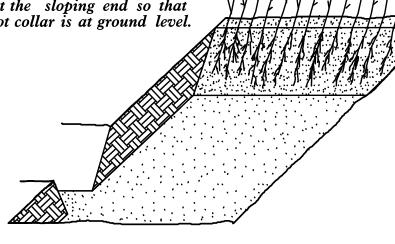
HEALING IN

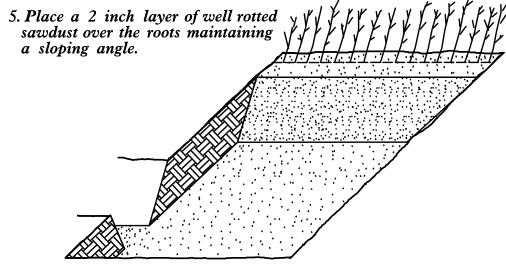
- 1. Locate a healing-in site in a shady, well protected area.
- 2. Excavate a flat bottom trench 12 inches deep and provide drainage.





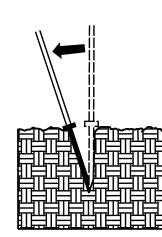
4. Place a single layer of plants against the sloping end so that the root collar is at ground level.



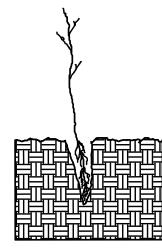


6. Repeat layers of plants and sawdust as necessary and water thoroughly.

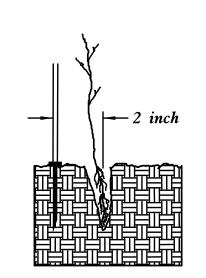
DIBBLE PLANTING METHOD USING THE KBC PLANTING BAR



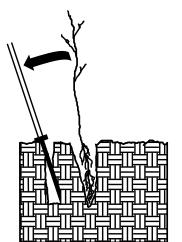
I. Insert planting bar as shown and pull handle toward planter.



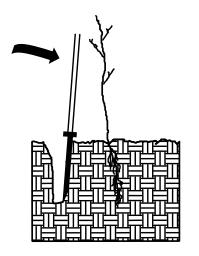
2. Remove planting bar and place seedling at correct depth.



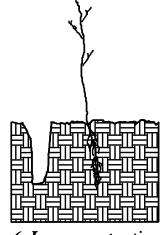
3. Insert planting bar 2 inches toward planter from seedling.



4. Pull handle of bar toward planter, firming soil at bottom.



5. Push handle forward firming soil at top.



6. Leave compaction hole open. Water thoroughly.

PLANTING NOTES:

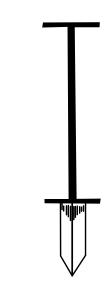
PLANTING BAG

During planting, seedlings shall be kept in a moist canvas bag or similar container to prevent the root systems from drying.



KBC PLANTING BAR
Planting bar shall have a
blade with a triangular
cross section, and shall
be 12 inches long,
4 inches wide and
1 inch thick at center.

ROOT PRUNING
All seedlings shall be root
pruned, if necessary, so that
no roots extend more than
10 inches below the root collar.



STATE	STATE	SHEET NO.	TOTAL SHEETS	
N.C.	17BP.5.R.77		RF-1	
STATE PROJ.NO.		F. A. PROJ. NO.	DESCRIPT	ION

REFORESTATION

☐ TREE REFORESTATION SHALL BE PLANTED 6 FT. TO 10 FT. ON CENTER, RANDOM SPACING, AVERAGING 8 FT. ON CENTER, APPROXIMATELY 680 PLANTS PER ACRE.

REFORESTATION

MIXTURE, TYPE, SIZE, AND FURNISH SHALL CONFORM TO THE FOLLOWING:

33% LIRIODENDRON TULIPIFERA TULIP POPLAR 12 in - 18 in BR AMERICAN SYCAMORE 33% PLATANUS OCCIDENTALIS 12 in - 18 in BR

34% BETULA NIGRA RIVER BIRCH 12 in - 18 in BR

REFORESTATION DETAIL SHEET

N.C.D.O.T. – ROADSIDE ENVIRONMENTAL UNIT

T.I.P. NO.

17BP.5.R.77

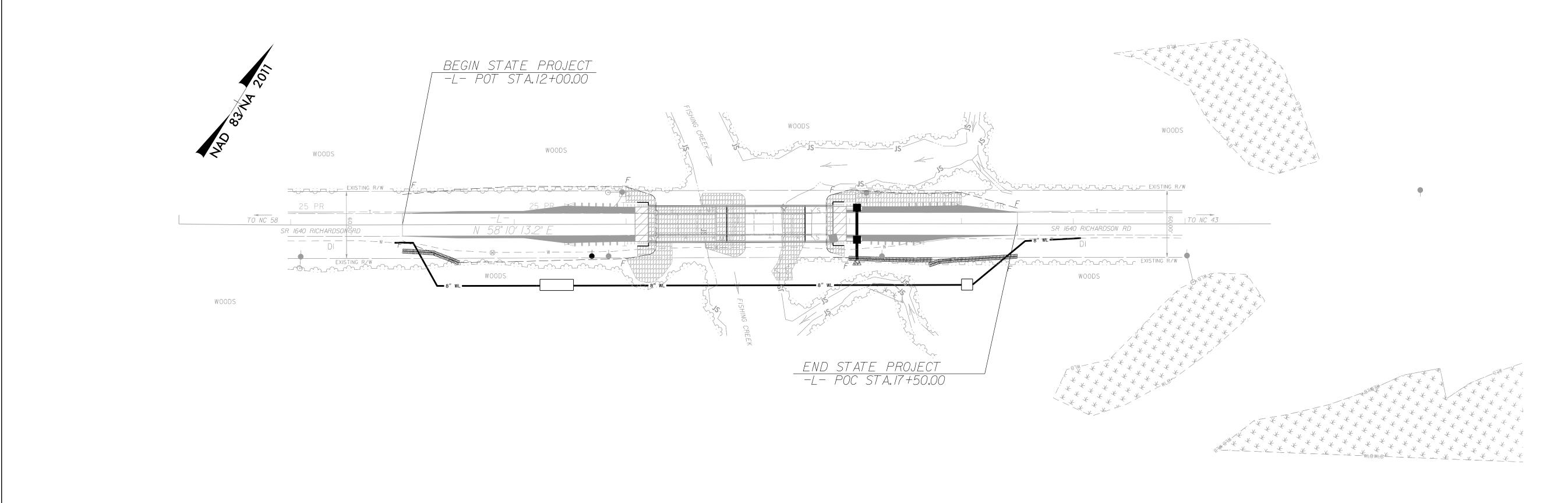
UO-1

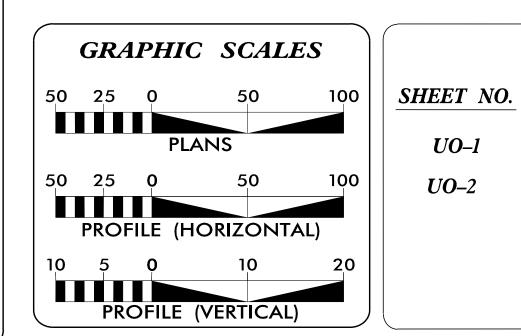
STATE OF NORTH CAROLINA

UTILITIES BY OTHERS PLANS WARREN COUNTY

LOCATION: BRIDGE NO. 77 OVER FISHING CREEK ON SR 1640 (RICHARDSON RD)

TYPE OF WORK: POWER RELOCATION, WATERLINE RELOCATION AND TELECOMMUNICATIONS REMOVAL





UO-1

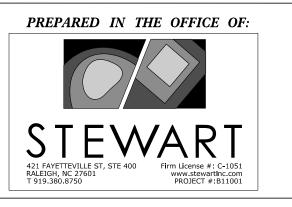
UO–2

INDEX OF SHEETS **DESCRIPTION** TITLE SHEET

UTILITY BY OTHERS PLAN SHEET

UTILITY OWNERS ON PROJECT

(A) WARREN COUNTY – WATER (B) CENTURY LINK - TELECOMMUNICATIONS (C) HALIFAX ELECTRIC – POWER



DAVID RUGGLES, PE ELIZABETH PHELPS, EI

PROJECT ENGINEER PROJECT DESIGN ENGINEER

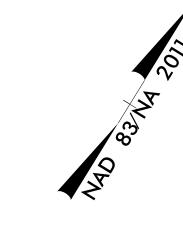
PROJECT REFERENCE NO. SHEET NO. 17RP 5 R 77 LIO-2

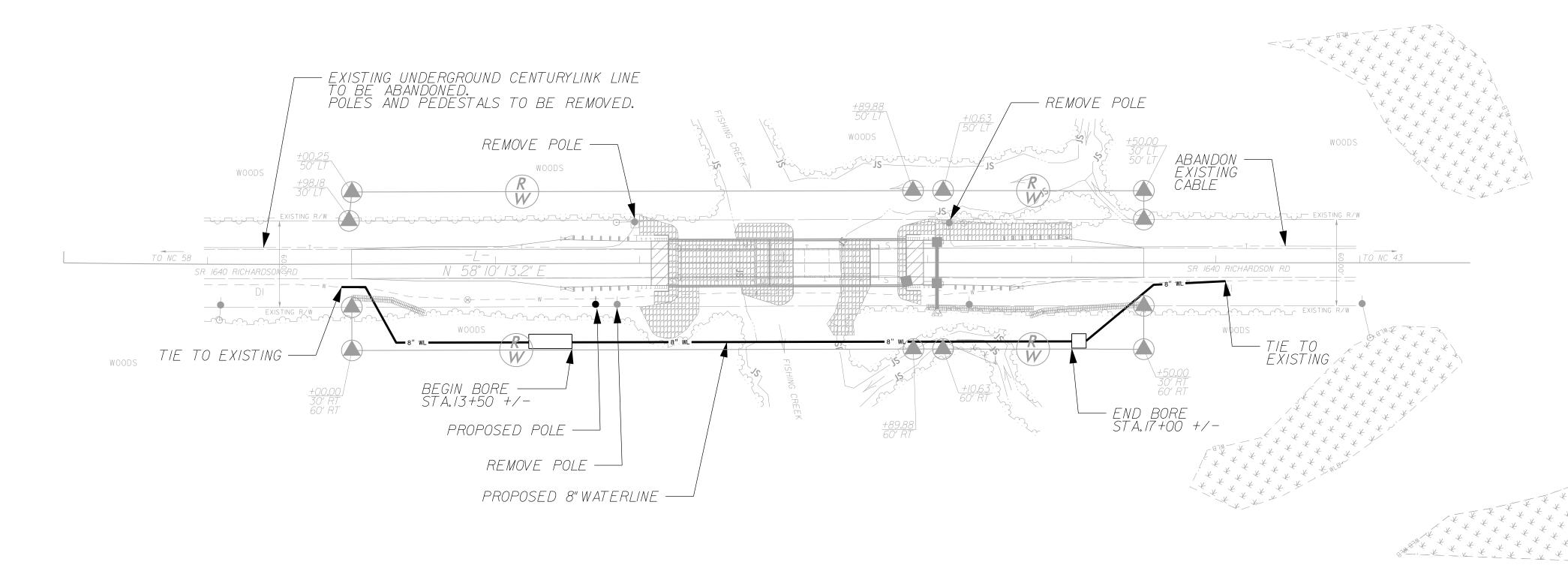
17BP.5.R.77 UC

UTILITIES BY OTHERS

NOTE:
ALL PROPOSED UTILITY WORK
SHOWN ON THIS SHEET WILL
BE DONE BY OTHERS







NOTES:

DESIGN STANDARDS IN SENSITIVE WATERSHEDS [15A NCAC 04B.0124 (b)-(e)] ARE INCORPORATED INTO NCDOT PROJECTS THAT OCCUR WITHIN OR UPSTREAM OF WATER BODIES THAT CONTAIN FEDERALLY PROTECTED AQUATIC SPECIES.

UBO CONTRACTOR AND INSPECTOR WILL VISUALLY MONITOR FISHING CREEK AND UNNAMED TRIBUTARIES TO FISHING CREEK DURING BORE OPERATIONS. THE CONTRACTOR WILL BE REQUIRED TO STOP ACTIVITY IF THERE IS ANY VISIBLE LOSS OF BORE FLUIDS IN THE STREAM CHANNEL AND THE INSPECTOR MUST IMMEDIATELY CONTACT THE RESIDENT ENGINEER AND THE DIVISION ENVIRONMENTAL SUPERVISOR FOR GUIDANCE ON HOW TO PROCEED WITH THE OPERATION.

SOME TREE CLEARING MAY BE DONE UNDER AND ADJACENT TO THE EXISTING OVERHEAD POWER LINE LOCATED TO THE EAST AND WEST OF SR 1640; HOWEVER, THERE WILL BE NO GRUBBING OF TREES LOCATED ALONG THE BANKS OF FISHING CREEK.

PROJECT REFERENCE NO. 17BP.5.R.77 X-/A

STATE OF NORTH CAROLINA **DIVISION OF HIGHWAYS**

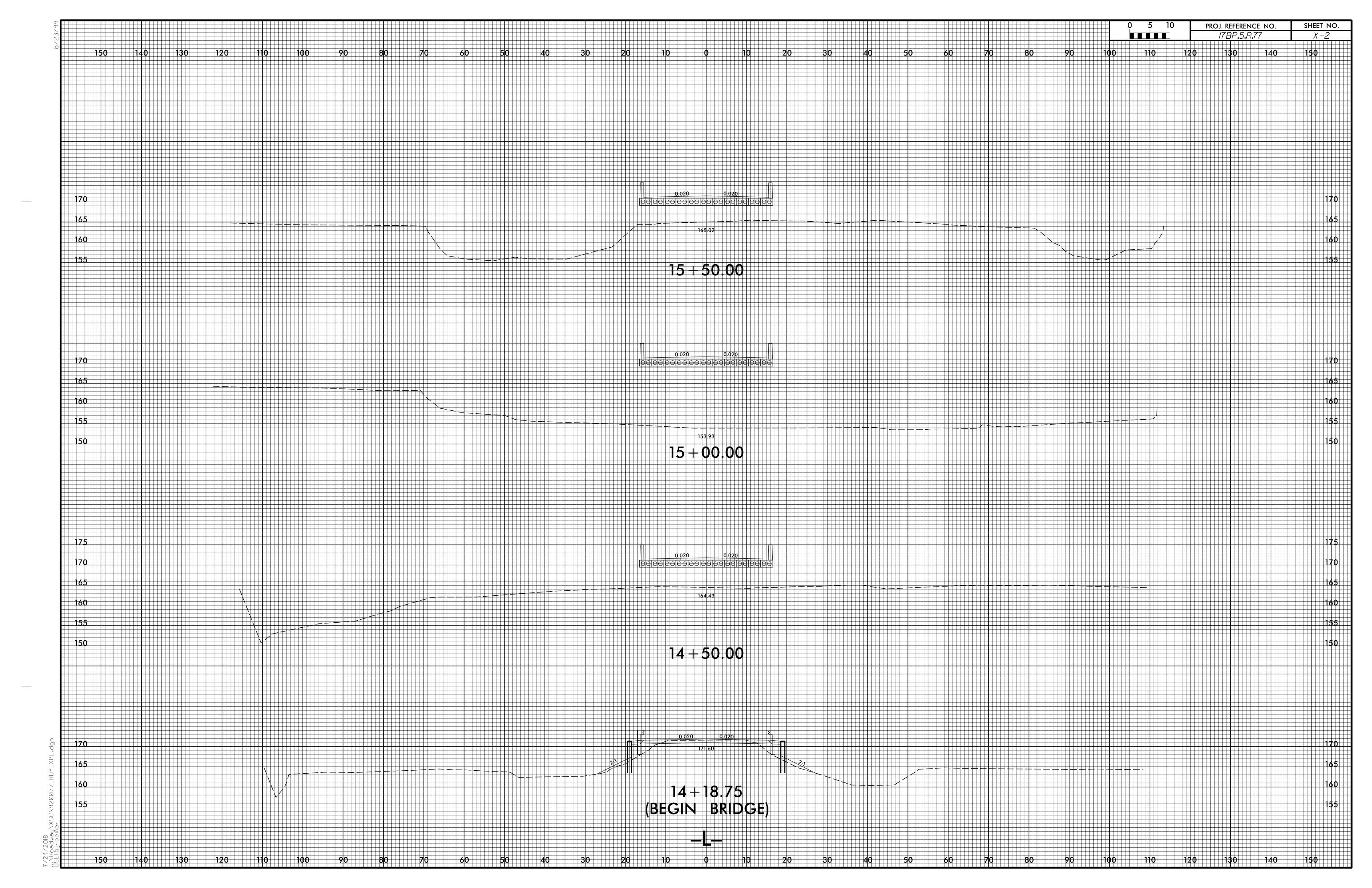
NOTE: EMBANKMENT COLUMN DOES NOT INCLUDE BACKFILL FOR UNDERCUT

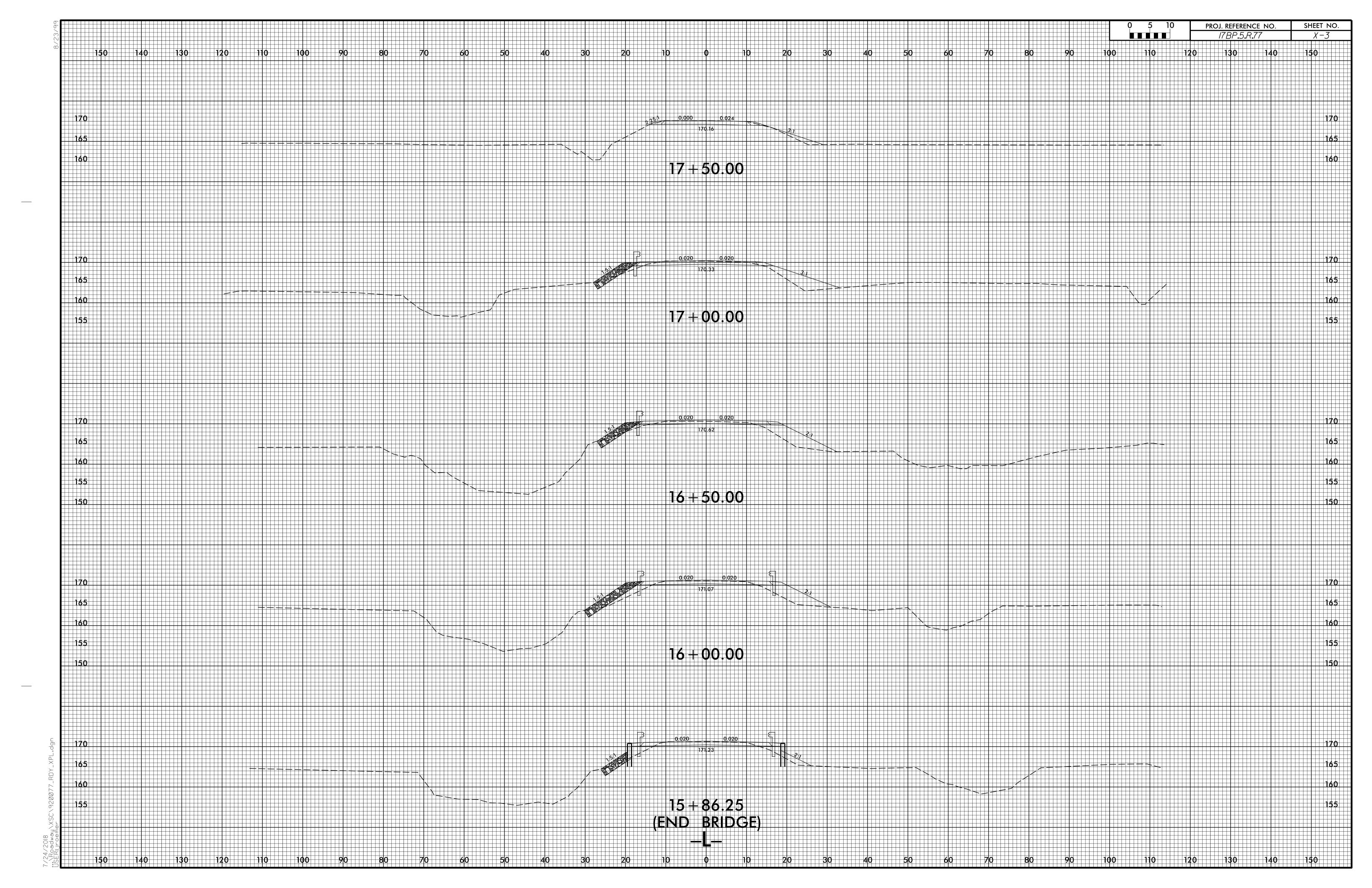
CROSS-SECTION SUMMARY

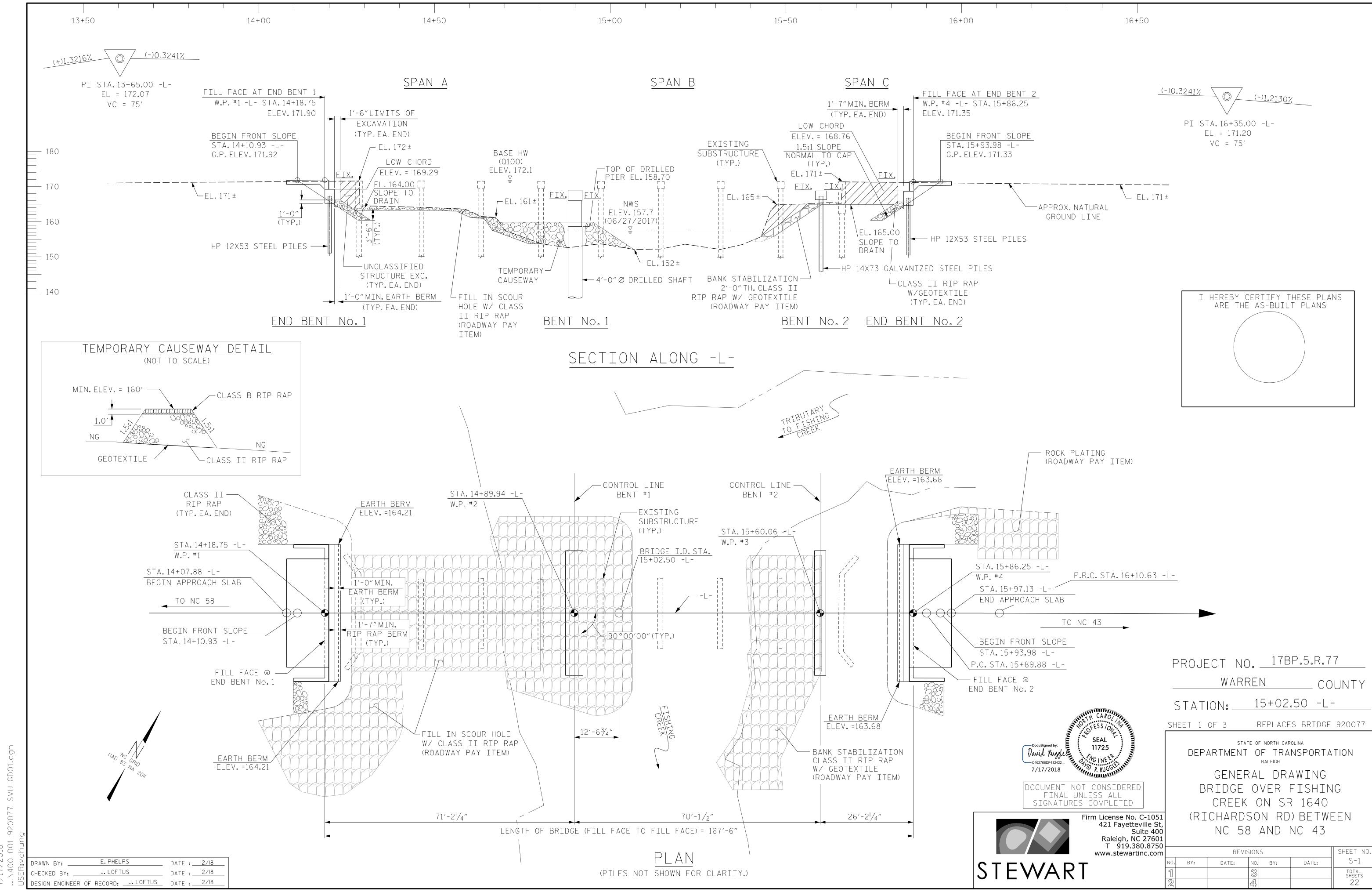
Uncl. Exc.	Embt
(cu. yd.)	(cu. yd.)
0	0
37	66
26	121
12	174
12	185
10	44
	(cu. yd.) 0 37 26 12

Station	Uncl. Exc.	Embt
-L-	(cu. yd.)	(cu. yd.)
15+86.75	0	0
16+00.00	14	19
16+50.00	40	85
17+00.00	44	77
17+50.00	45	44

Approximate quantities only. Unclassified excavation, borrow
excavation, fine grading, clearing and grubbing and removal of
existing pavement will be paid for at the lump sum price for
"Grading".







WARREN 77

/17/2018

FOUNDATION LAYOUT

ALL BENTS ARE PARALLEL

FOUNDATION NOTES

- 1) FOR PILES, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.
- 2) PILES AT END BENT NO.1 ARE DESIGNED FOR A FACTORED RESISTANCE OF 85 TONS PER PILE.
- 3) PILES AT BENT NO. 2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 120 TONS PER PILE.
- 4) PILES AT END BENT NO. 2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 50 TONS PER PILE.
- 5) DRIVE PILES AT END BENT NO.1 TO A REQUIRED DRIVING RESISTANCE OF 145 TONS PER PILE.
- 6) DRIVE PILES AT BENT NO.2 TO A REQUIRED DRIVING RESISTANCE OF 200 TONS PER PILE.
- 7) DRIVE PILES AT END BENT NO.2 TO A REQUIRED DRIVING RESISTANCE OF 85 TONS PER PILE.
- 8) INSTALL PILES AT BENT NO.2 TO A TIP ELEVATION NO HIGHER THAN 146 FT.
- 9) THE SCOUR CRITICAL ELEVATION FOR BENT NO.2 IS ELEVATION 161 FT. SCOUR CRITICAL ELEVATIONS ARE USED TO MONITOR POSSIBLE SCOUR PROBLEMS DURING THE LIFE OF THE STRUCTURE.
- 10) FOR DRILLED PIERS, SEE SECTION 411 OF THE STANDARD SPECIFICATIONS.
- 11) DRILLED PIERS AT BENT NO.1 ARE DESIGNED FOR A FACTORED RESISTANCE OF 425 TONS PER PIER. CHECK FIELD CONDITIONS FOR THE REQUIRED TIP RESISTANCE OF 65 TSF.
- 12) INSTALL DRILLED PIERS AT BENT NO.1 TO A TIP ELEVATION NO HIGHER THAN 122 FT. WITH THE REQUIRED TIP RESISTANCE AND A PENETRATION OF AT LEAST 6 FT INTO ROCK AS DEFINED BY ARTICLE 411-1 OF THE STANDARD SPECIFICATIONS.
- 13) THE SCOUR CRITICAL ELEVATION FOR BENT NO.1 IS ELEVATION 140 FT. SCOUR CRITICAL ELEVATIONS ARE USED TO MONITOR POSSIBLE SCOUR PROBLEMS DURING THE LIFE OF THE STRUCTURE.
- 14) PERMANENT STEEL CASINGS ARE REQUIRED FOR DRILLED PIERS AT BENT NO.1.DO NOT EXTEND PERMANENT CASINGS BELOW ELEVATION 128 FT WITHOUT PRIOR APPROVAL FROM THE ENGINEER.
- 15) SID INSPECTIONS MAY BE REQUIRED FOR DRILLED PIERS. THE ENGINEER WILL DETERMINE THE NEED FOR SID INSPECTIONS. FOR SID INSPECTIONS, SEE SECTION 411 OF THE STANDARD SPECIFICATIONS.
- 16) CSL TUBES ARE REQUIRED AND CSL TESTING MAY BE REQUIRED FOR DRILLED PIERS. THE ENGINEER WILL DETERMINE THE NEED FOR CSL TESTING. FOR CSL TESTING, SEE SECTION 411 OF THE STANDARD SPECIFICATIONS.
- 17) SPT MAY BE REQUIRED FOR DRILLED PIERS. THE ENGINEER WILL DETERMINE THE NEED FOR SPT. FOR SPT TESTING, SEE SECTION 411 OF THE STANDARD SPECIFICATIONS.



STEWART

Raleigh, NC 27601 T 919.380.8750

www.stewartinc.com

PROJECT NO. 17BP.5.R.77

WARREN COUNTY

STATION: 15+02.50 -L
SHEET 2 OF 3

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

ONSIDERED

SS ALL

DMPLETED

License No. C-1051
421 Fayetteville St,
Suite 400

REVISIONS

NO. BY: DATE: NO. BY: DATE: S-2

TOTAL SHEETS

22

6/29/2018 ...\400_002_920077_SMU_FL02 USER:ephelps

DRAWN BY: _____E.PHELPS DATE : 2/18

CHECKED BY: _____J.LOFTUS DATE : 2/18

DESIGN ENGINEER OF RECORD: ____J.LOFTUS DATE : 2/18

GENERAL NOTES:

- 1. ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING
- 2. THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS
- 3. THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1
- THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH "HEC 18 EVALUATING SCOUR AT
- THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCE BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.
- REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED SO AS NOT TO ALLOW DEBRIS TO FALL INTO THE WATER. THE CONTRACTOR SHALL REMOVE THE BRIDGE AND SUBMIT PLANS FOR DEMOLITION IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.
- 7. FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.
- 8. FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.
- 9. FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.
- 10. FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.
- 11. FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.
- 12. FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.
- 13. ASPHALT WEARING SURFACE IS INCLUDED IN ROADWAY QUANTITY ON ROADWAY PLANS.
- 14. THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA SHALL BE EXCAVATED FOR A DISTANCE OF 25'± FT LEFT SIDE AND 55'± RIGHT SIDE OF CENTERLINE ROADWAY AS DIRECTED BY THE ENGINEER. THIS WORK WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION. SEE SECTION 412 OF THE STANDARD SPECIFICATIONS
- 15. THE EXISTING STRUCTURE #920077 CONSISTING OF EIGHT (8) TIMBER JOISTS SPANS @ 17'-6", 17'-6", 17'-0", 17'-0", 17'-0", 17'-3" & 17'-3" (137'-6" TOTAL LENGTH), 19'-2" CLEAR ROADWAY WIDTH AND TIMBER DECK WITH AN ASPHALT WEARING SURFACE ON TIMBER END BENTS & BENTS WITH TIMBER PILES ENCASED WITH CONCRETE NEAR THE WATER LINE AND LOCATED AT THE PROPOSED STRUCTURE SHALL BE REMOVED IN THEIR ENTIRETY. THE EXISTING BRIDGE IS PRESENTLY POSTED FOR A LOAD LIMIT. SHOULD THE STRUCTURAL INTEGRITY OF THE BRIDGE DETERIORATE DURING CONSTRUCTION OF THE PROPOSED BRIDGE, A LOAD LIMIT MAY BE POSTED AND MAY BE REDUCED AS FOUND NECESSARY DURING THE LIFE OF THE PROJECT.
- 16. FOR ASBESTOS ASSESSMENT FOR BRIDGE DEMOLITION AND RENOVATION ACTIVITIES, SEE SPECIAL
- 17. NO HEAVY EQUIPMENT WILL BE PLACED IN FISHING CREEK.
- 18. BEST MANAGEMENT PRACTICES FOR BRIDGE DEMOLITION AND REMOVAL WILL BE IMPLEMENTED DURING THE REMOVAL OF THE EXISTING BRIDGE.

									TOTAL	BILL	OF	MATER	RIAL -								
	CONSTRUCTION MAINTENANCE & REMOVAL OF TEMPORARY ACCESS	REMOVAL OF ASBESTOS ASSESSMENT	4'-0"DIA. DRILLED PIERS NOT IN SOIL	4'-0"DIA. DRILLED PIERS IN SOIL	PERMANENT STEEL CASING FOR 4'-0"Ø DRILLED PIER	SID INSPECTIONS	SPT TESTING	CSL TESTING	UNCLASSIFIED STRUCTURE EXCAVATION	CLASS A CONCRETE	BRIDGE APPROACH SLABS	REINFORCING STEEL	SPIRAL COLUMN REINFORCING STEEL	PILE DRIVING EQUIPMENT SETUP FOR HP 12X53 STEEL PILES	PILE DRIVING EQUIPMENT SETUP FOR HP 14X73 GALVANIZED STEEL PILES	HP 12X53 GALVANIZED STEEL STEEL PILES PILES	VERTICAL CONCRETE BARRIER RAIL	CLASS II	ELASTOMERIC BEARINGS	GEOTEXTILE FOR DRAINAGE	3'-0" × 2'-0" PRESTRESSED CORED SLABS
	LUMP SUM	LUMP SUM LUMP SUM	LIN.FT.	LIN.FT.	LIN.FT.	EACH	EACH	EACH	LUMP SUM	CY	LUMP SUM	LBS	LBS	EACH	EACH	No. LF No. LF	LF	TON	SY	LUMP SUM	No. LF
SUPERSTRUCTURE											LUMP SUM						330.5			LUMP SUM	33 1815.00
END BENT No.1									LUMP SUM	21.8		2,636		7		7 175		77	85		
BENT No.1			25	85.1	92.1	1	1	1		24.3		13,778	4,107								
BENT No. 2										10.7		2,136			8	8 240					
END BENT No. 2									LUMP SUM	21.8		2,636		7		7 175		68	75		
TOTAL	LUMP SUM	LUMP SUM LUMP SUM	25	85.1	92.1	1	1	1	LUMP SUM	78.6	LUMP SUM	21,186	4,107	14	8	14 350 8 240	330.5	145	160	LUMP SUM	33 1815.00

GENERAL NOTES (CONTINUED):

- 19. THE BRIDGE WILL BE REMOVED FROM THE TOP DOWN, FIRST REMOVING THE ASPHALT WITH CONTAINMENT MEASURES IN PLACE TO PREVENT COMPONENTS OF THE BRIDGE DECK FROM DROPPING INTO THE STREAM. THE METHOD OF CONTAINMENT WILL BE PROPOSED BY THE CONTRACTOR AND APPROVED BY THE ENGINEER. THIS WILL BE FOLLOWED BY REMOVAL OF THE DECKING, GIRDERS, ETC., AND FINALLY THE WOODEN PILES. AN ATTEMPT WILL BE MADE TO COMPLETELY REMOVE THE EXISTING AND HISTORIC TIMBER PILES ALONG THE WATER'S EDGE (WITH CONCRETE ENCASEMENTS WHERE APPLICABLE) BEHIND AN IMPERVIOUS DIKE (THAT IS INCIDENTAL TO THE WORK). AN ATTEMPT WILL ALSO BE MADE TO COMPLETELY REMOVE THE INTERIOR EXISTING AND HISTORIC TIMBER PILES. IF THE REMOVAL OF THE TIMBER PILES CANNOT BE ACCOMPLISHED WITH MINIMAL SUBSTRATE DISTURBANCE, THE PILES WILL BE PINCHED OFF ONE FOOT BELOW THE MUD LINE OR CUT FLUSH WITH EXISTING MUD LINE AS DIRECTED BY THE ENGINEER. THE CONTRACTOR WILL NOT BE ALLOWED TO DRAG REMOVED TIMBER PILES ON OR ACROSS THE STREAM BED.
- 20. DECK DRAINS WILL NOT BE ALLOWED TO DISCHARGE DIRECTLY INTO THE STREAM.
- 21. AT THE CONTRACTOR'S OPTION, AND UPON REMOVAL OF THE CAUSEWAY, THE CLASS II RIP RAP USED IN THE CAUSEWAY MAY BE PLACED AS RIP RAP SLOPE PROTECTION. SEE SPECIAL PROVISIONS FOR CONSTRUCTION, MAINTENANCE AND REMOVAL OF TEMPORARY ACCESS AT STATION 15+02.50 -L-.
- 22. FOR REMOVAL OF EXISTING STRUCTURE AT STATION 15+02.50 -L-, SEE SPECIAL PROVISIONS.

E. PHELPS DRAWN BY: DATE : 2/18 J. LOFTUS DATE : ___2/18 DESIGN ENGINEER OF RECORD: __J.LOFTUS__ DATE : __2/18_

HYDRAULIC DATA

172.0 FT

DESIGN DISCHARGE 9700 CFS FREQUENCY OF DESIGN FLOOD 25 YR. DESIGN HIGHWATER ELEV. 169.8 FT. DRAINAGE AREA 118 SQ. MI. BASE DISCHARGE (Q100) 15053 C.F.S

OVERTOPPING FLOOD DATA

BASE HIGHWATER ELEV.

OVERTOPPING DISCHARGE 8040 CFS FREQUENCY OF OVERTOPPING FLOOD 10+ YR. OVERTOPPING FLOOD ELEV. * 169.4 FT.

* OVERTOPPING AT SAG ±296 FT. PAST END OF PROJECT



FINAL UNLESS ALL SIGNATURES COMPLETED



Firm License No. C-105: 421 Fayetteville St Suite 400 Raleigh, NC 27601 T 919.380.8750 www.stewartinc.com

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION GENERAL DRAWING

15+02.50 -L-

PROJECT NO. <u>17</u>BP.5.R.77

COUNTY

WARREN

STATION: _

SHEET 3 OF 3

BRIDGE OVER FISHING CREEK ON SR 1640 (RICHARDSON RD) BETWEEN NC 58 AND NC 43

	REVIS	SION	IS		SHEET NO.
BY:	DATE:	NO.	BY:	DATE:	S-3
		3			TOTAL SHEETS
		4			22

										STRE	NGTH	I LIN	MIT ST	ATE				SE	ERVICE	- III	LIMI	T STA	ATE	
										MOMENT					SHEAR						MOMENT			
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING	MINIMUM RATING FACTORS (RF)	TONS = W X RF	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (++)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (++)	COMMENT NUMBER
		HL-93(Inv)	N/A	1	1.006		1.75	0.273	1.03	70′	EL	34.5	0.507	1.32	70′	EL	6.9	0.80	0.273	1.01	70′	EL	34.5	
DESIGN		HL-93(0pr)	N/A		1.341		1.35	0.273	1.34	70′	EL	34.5	0.507	1.72	70′	EL	6.9	N/A						
LOAD RATING		HS-20(Inv)	36.000	2	1.306	47.02	1.75	0.273	1.34	70′	EL	34.5	0.507	1.65	70′	EL	6.9	0.80	0.273	1.31	70′	EL	34.5	
IVATINO		HS-20(0pr)	36.000		1.74	62.64	1.35	0.273	1.74	70′	EL	34.5	0.507	2.14	70′	EL	6.9	N/A						
		SNSH	13.500		2.917	39.379	1.4	0.273	3.75	70′	EL	34.5	0.507	4.87	70′	EL	6.9	0.80	0.273	2.92	70′	EL	34.5	
		SNGARBS2	20.000		2.187	43.741	1.4	0.273	2.81	70′	EL	34.5	0.507	3.47	70′	EL	6.9	0.80	0.273	2.19	70′	EL	34.5	
		SNAGRIS2	22.000		2.077	45.69	1.4	0.273	2.67	70′	EL	34.5	0.507	3.23	70′	EL	6.9	0.80	0.273	2.08	70′	EL	34.5	
		SNCOTTS3	27.250		1.452	39.565	1.4	0.273	1.87	70′	EL	34.5	0.507	2.43	70′	EL	6.9	0.80	0.273	1.45	70′	EL	34.5	
	S	SNAGGRS4	34.925		1.218	42.554	1.4	0.273	1.57	70′	EL	34.5	0.507	2.03	70′	EL	6.9	0.80	0.273	1.22	70′	EL	34.5	
		SNS5A	35.550		1.191	42.346	1.4	0.273	1.53	70′	EL	34.5	0.507	2.06	70′	EL	6.9	0.80	0.273	1.19	70′	EL	34.5	
		SNS6A	39.950		1.095	43.747	1.4	0.273	1.41	70′	EL	34.5	0.507	1.88	70′	EL	6.9	0.80	0.273	1.10	70′	EL	34.5	
LEGAL		SNS7B	42.000		1.043	43.801	1.4	0.273	1.34	70′	EL	34.5	0.507	1.85	70′	EL	6.9	0.80	0.273	1.04	70′	EL	34.5	
LOAD RATING		TNAGRIT3	33.000		1.336	44.087	1.4	0.273	1.72	70′	EL	34.5	0.507	2.23	70′	EL	6.9	0.80	0.273	1.34	70′	EL	34.5	
I MATERIO		TNT4A	33.075		1.342	44.401	1.4	0.273	1.72	70′	EL	34.5	0.507	2.17	70′	EL	6.9	0.80	0.273	1.34	70′	EL	34.5	
		TNT6A	41.600		1.1	45.746	1.4	0.273	1.41	70′	EL	34.5	0.507	1.98	70′	EL	6.9	0.80	0.273	1.10	70′	EL	34.5	
		TNT7A	42.000		1.106	46.462	1.4	0.273	1.42	70′	EL	34.5	0.507	1.94	70′	EL	6.9	0.80	0.273	1.11	70′	EL	34.5	
		TNT7B	42.000		1.147	48.18	1.4	0.273	1.47	70′	EL	34.5	0.507	1.8	70′	EL	6.9	0.80	0.273	1.15	70′	EL	34.5	
		TNAGRIT4	43.000		1.089	46.838	1.4	0.273	1.4	70′	EL	34.5	0.507	1.74	70′	EL	6.9	0.80	0.273	1.09	70′	EL	34.5	
		TNAGT5A	45.000		1.026	46.175	1.4	0.273	1.32	70′	EL	34.5	0.507	1.74	70′	EL	6.9	0.80	0.273	1.03	70′	EL	34.5	
		TNAGT5B	45.000	3	1.013	45.579	1.4	0.273	1.3	70'	EL	34.5	0.507	1.66	70′	EL	6.9	0.80	0.273	1.01	70′	EL	34.5	

LOAD FACTORS:

DESIGN	LIMIT STATE	$\gamma_{ extsf{DC}}$	$\gamma_{\sf DW}$
LOAD RATING	STRENGTH I	1.25	1.50
FACTORS	SERVICE III	1.00	1.00

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

(#) CONTROLLING LOAD RATING

1 DESIGN LOAD RATING (HL-93)

2 DESIGN LOAD RATING (HS-20)

(3) LEGAL LOAD RATING **

** SEE CHART FOR VEHICLE TYPE

GIRDER LOCATION

I - INTERIOR GIRDER

EL - EXTERIOR LEFT GIRDER

ER - EXTERIOR RIGHT GIRDER

LRFR SUMMARY

FOR SPAN 'A' AND SPAN 'B'

Docusigned by:

Pavid Ruggles

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL Signatures completed



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PROJECT NO. <u>178P.5.R.77</u> WARREN COUNTY STATION: ____15+02.50 -L-

SHEET 1 OF 2 STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION STANDARD

70' CORED SLAB UNIT 90° SKEW

(NON-INTERSTATE TRAFFIC)

SHEET NO. REVISIONS S-4 NO. BY: DATE: TOTAL SHEETS 22

										STRE	INGTH	I LIN	MIT ST	ATE				SE	ERVICE	III	LIMI	T STA	TE	
										MOMENT					SHEAR						MOMENT			
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING	MINIMUM RATING FACTORS (RF)	TONS = W X RF	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (++)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (++)	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (++)	COMMENT NUMBER
		HL-93(Inv)	N/A	1	2.42		1.75	0.370	2.42	25′	EL	12.5	0.400	2.98	25′	EL	22.2	0.80	0.370	3.30	25′	EL	12.5	
DESIGN		HL-93(0pr)	N/A		3.14		1.35	0.370	3.14	25′	EL	12.5	0.400	3.86	25′	EL	22.2	N/A						
LOAD Rating		HS-20(Inv)	36.000	2	3.62	130.32	1.75	0.370	3.62	25′	EL	9.5	0.400	3.44	25′	EL	22.2	0.80	0.370	4.94	25′	EL	9.5	
KATING		HS-20(0pr)	36.000		4.70	169.20	1.35	0.370	4.70	25′	EL	9.5	0.400	4.46	25′	EL	22.2	N/A						
		SNSH	13.500		6.69	90.31	1.4	0.370	6.69	25′	EL	12.5	0.400	7.94	25′	EL	22.2	0.80	0.370	7.29	25′	EL	9.5	
	SNGARBS2	20.000		6.26	125.20	1.4	0.370	6.26	25′	EL	12.5	0.400	6.38	25′	EL	22.2	0.80	0.370	6.82	25′	EL	12.5		
		SNAGRIS2	22.000		6.59	131.80	1.4	0.370	6.59	25′	EL	9.5	0.400	6.25	25′	EL	22.2	0.80	0.370	7.19	25′	EL	9.5	
		SNCOTTS3	27.250		3.43	93.47	1.4	0.370	3.43	25′	EL	12.5	0.400	4.01	25′	EL	22.2	0.80	0.370	3.74	25′	EL	12.5	
	> S	SNAGGRS4	34.925		3.43	119.79	1.4	0.370	3.43	25′	EL	12.5	0.400	3.89	25′	EL	22.2	0.80	0.370	3.73	25′	EL	12.5	
		SNS5A	35.550		3.29	116.96	1.4	0.370	3.29	25′	EL	12.5	0.400	4.06	25′	EL	22.2	0.80	0.370	3.59	25′	EL	12.5	
		SNS6A	39.950		3.09	123.44	1.4	0.370	3.09	25′	EL	12.5	0.400	3.90	25′	EL	22.2	0.80	0.370	3.37	25′	EL	12.5	
LEGAL		SNS7B	42.000	3	3.05	128.10	1.4	0.370	3.05	25′	EL	12.5	0.400	3.93	25′	EL	22.2	0.80	0.370	3.33	25′	EL	12.5	
LOAD		TNAGRIT3	33.000		4.30	141.90	1.4	0.370	4.30	25′	EL	12.5	0.400	4.79	25′	EL	22.2	0.80	0.370	4.68	25′	EL	12.5	
RATING		TNT4A	33.075		3.78	125.02	1.4	0.370	3.78	25′	EL	12.5	0.400	4.35	25′	EL	22.2	0.80	0.370	4.12	25′	EL	12.5	
		TNT6A	41.600		3.55	147.68	1.4	0.370	3.55	25′	EL	12.5	0.400	4.08	25′	EL	22.2	0.80	0.370	3.86	25′	EL	12.5	
		TNT7A	42.000		3.66	153.72	1.4	0.370	3.66	25′	EL	12.5	0.400	4.07	25′	EL	22.2	0.80	0.370	3.99	25′	EL	12.5	
		TNT7B	42.000		3.32	139.44	1.4	0.370	3.32	25′	EL	9.5	0.400	3.92	25′	EL	22.2	0.80	0.370	3.62	25′	EL	12.5	
		TNAGRIT4	43.000		3.53	151.79	1.4	0.370	3.53	25′	EL	12.5	0.400	3.94	25′	EL	22.2	0.80	0.370	3.85	25′	EL	12.5	
		TNAGT5A	45.000		3.53	158.85	1.4	0.370	3.53	25′	EL	12.5	0.400	3.99	25′	EL	22.2	0.80	0.370	3.85	25′	EL	12.5	
		TNAGT5B	45.000		3.43	154.35	1.4	0.370	3.43	25′	EL	9.5	0.400	3.63	25′	EL	22.2	0.80	0.370	3.75	25′	EL	9.5	

LOAD FACTORS:

DESIGN	LIMIT STATE	$\gamma_{ extsf{DC}}$	$\gamma_{\sf DW}$
LOAD RATING	STRENGTH I	1.25	1.50
FACTORS	SERVICE III	1.00	1.00

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

(#) CONTROLLING LOAD RATING

1 DESIGN LOAD RATING (HL-93)

2 DESIGN LOAD RATING (HS-20)

(3) LEGAL LOAD RATING **

** SEE CHART FOR VEHICLE TYPE

GIRDER LOCATION

I - INTERIOR GIRDER

EL - EXTERIOR LEFT GIRDER

ER - EXTERIOR RIGHT GIRDER

SHEET 2 OF 2

<u>LRFR SUMMARY</u>

Docusigned by:

David Rugul

C462768DF412422...

7/17/2018

CARO

R. RUGGESS

R. RUGGESTAND

R.

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STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION

PROJECT NO. 17BP.5.R.77

STATION: ____15+02.50 -L-

COUNTY

WARREN

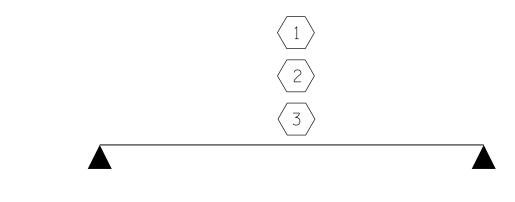
LRFR SUMMARY FOR

25' CORED SLAB UNIT 90° SKEW (NON-INTERSTATE TRAFFIC)

REVISIONS

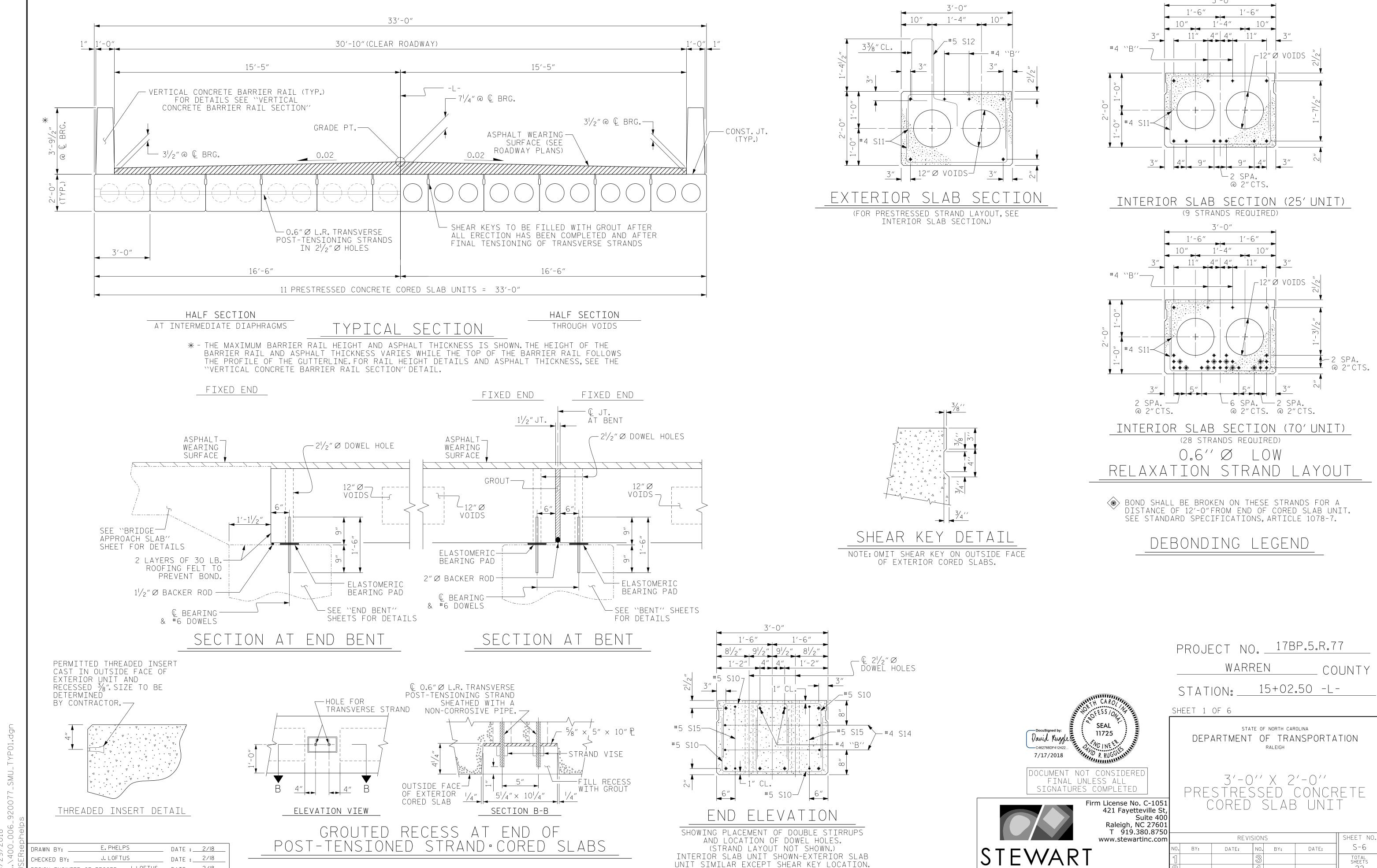
BY: DATE: NO. BY: DATE: S-5

3 TOTAL SHEETS
22



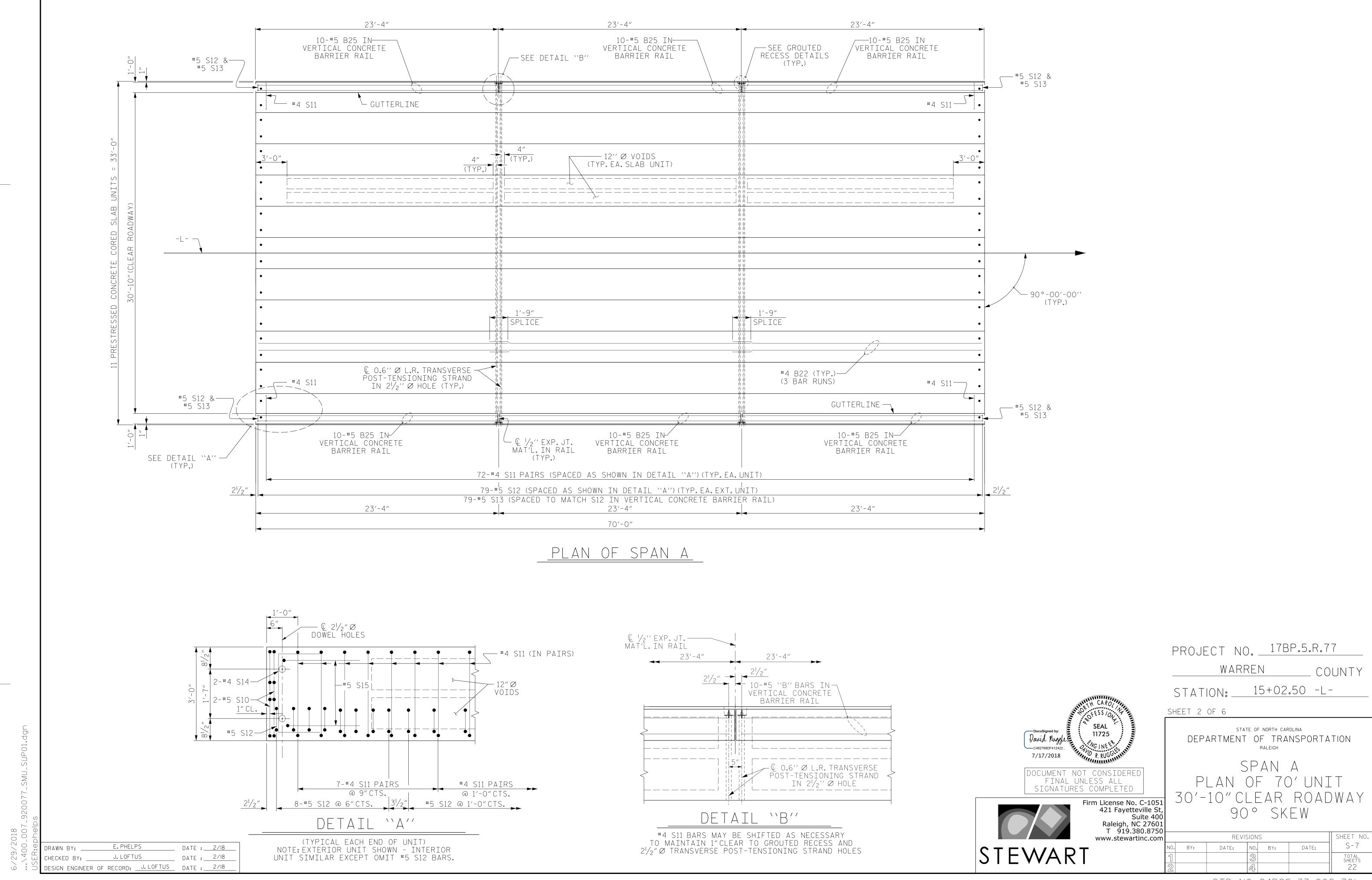
FOR SPAN 'C'

6/23/2018 ...\400_005_920077_SMU_LRFF ||SFR:@phelds

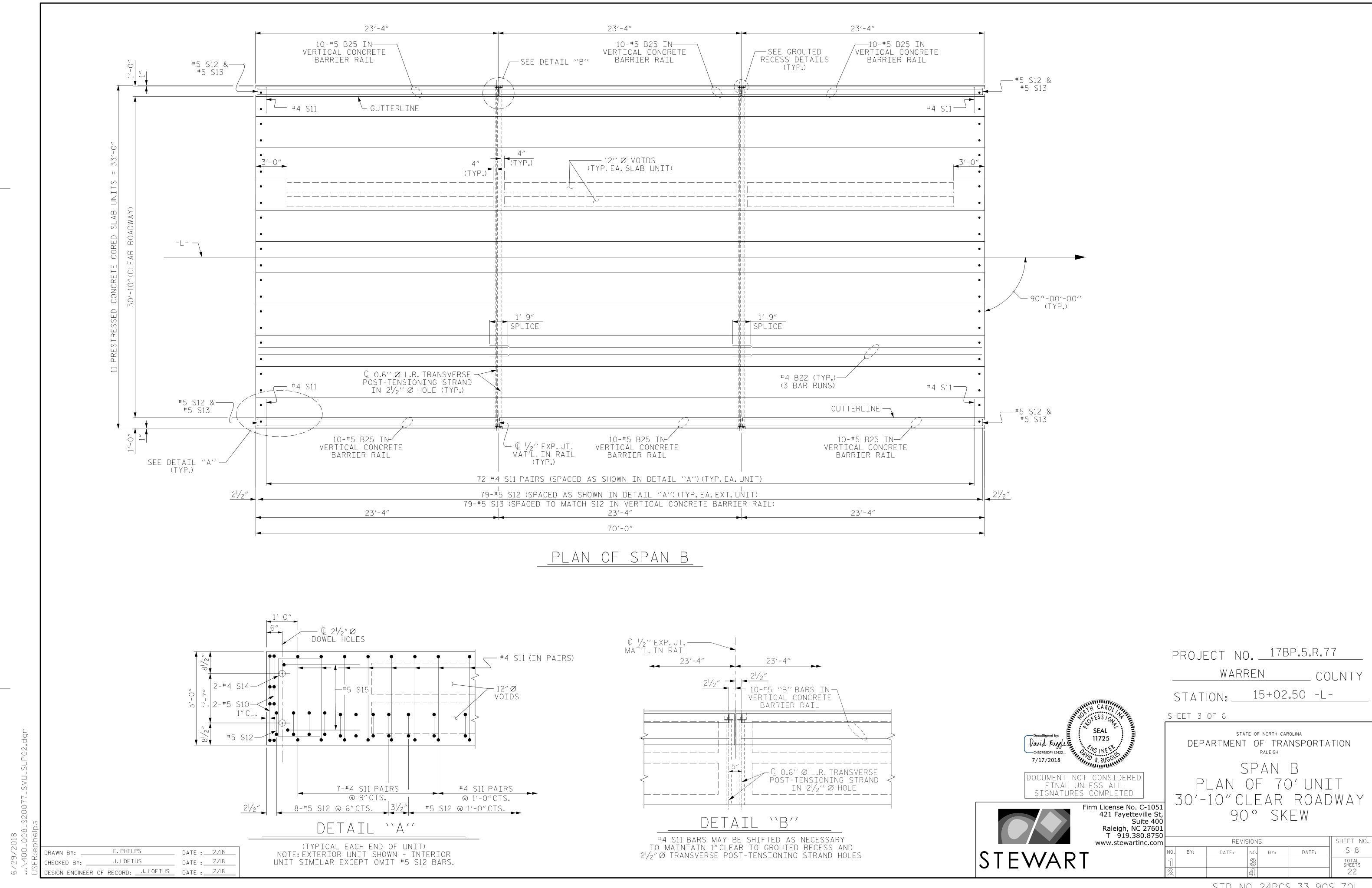


UNIT SIMILAR EXCEPT SHEAR KEY LOCATION.

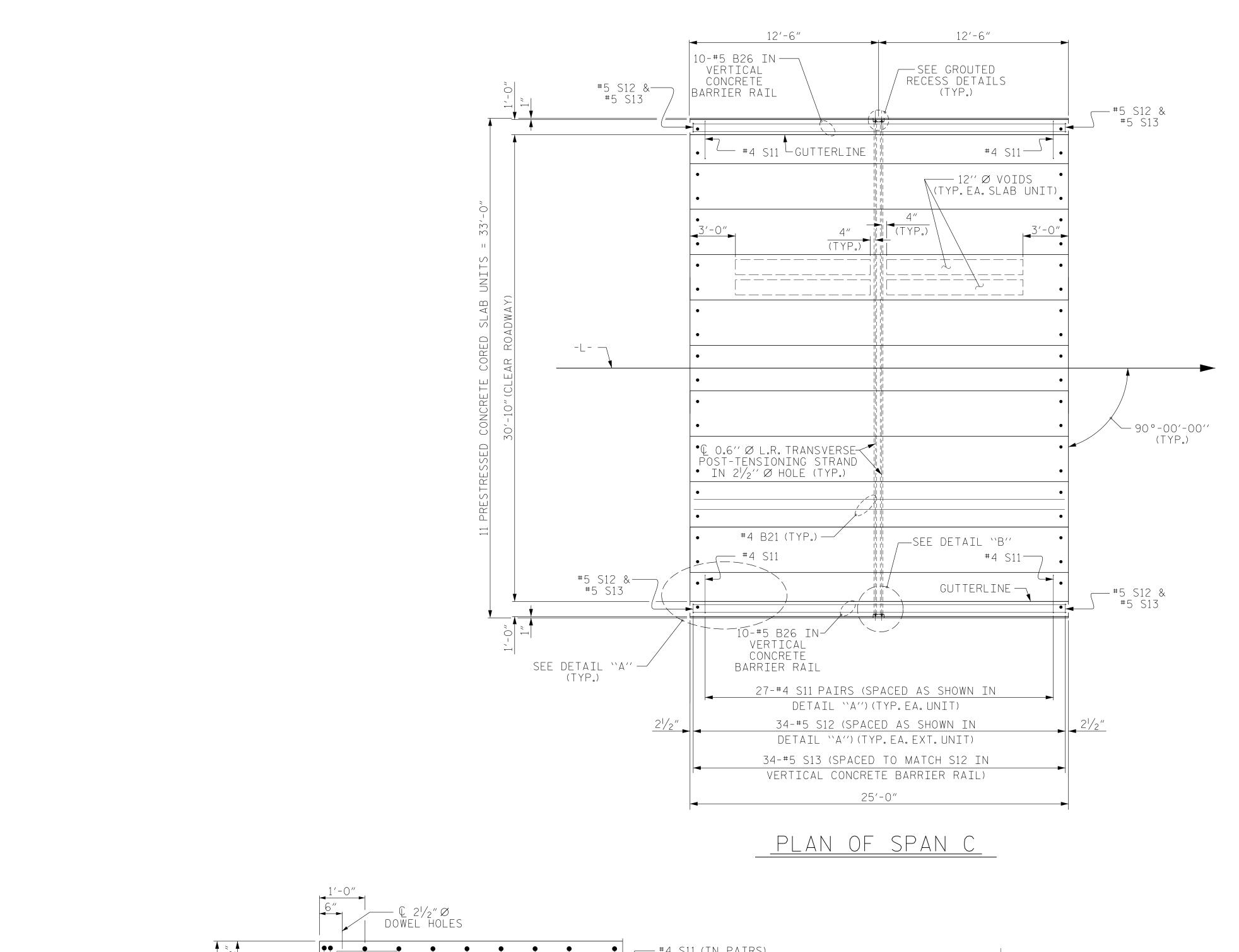
DESIGN ENGINEER OF RECORD: __J.LOFTUS__ DATE:___2/18_

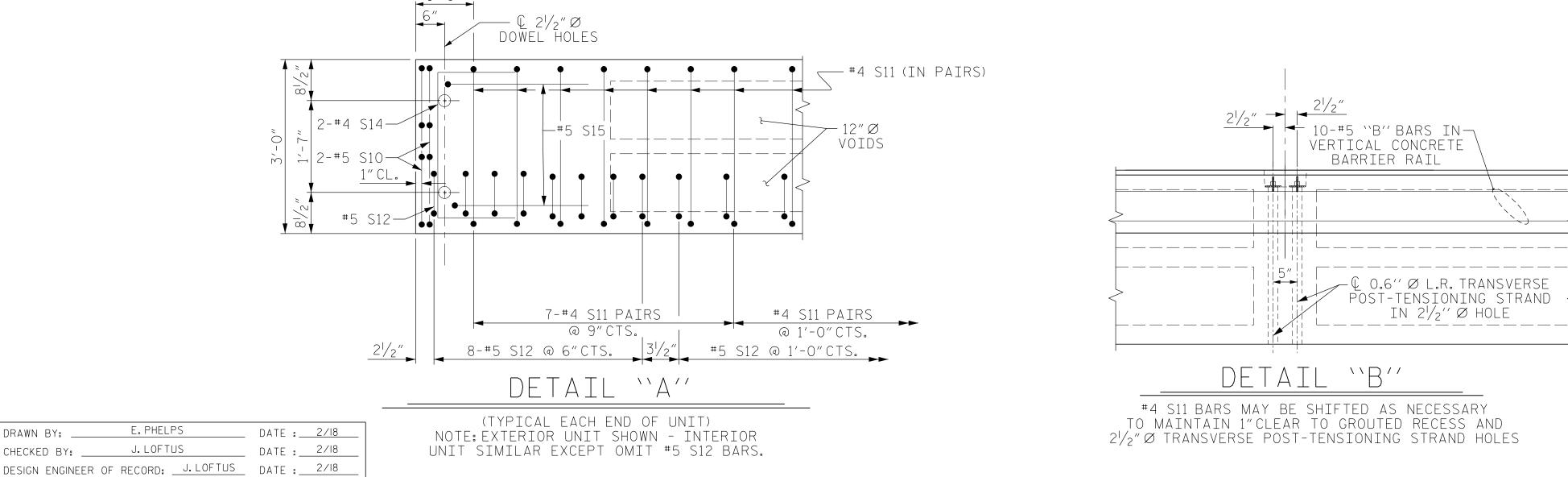


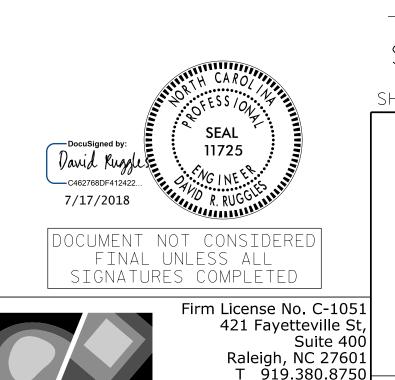
STD. NO. 24PCS_33_90S_70L



STD. NO. 24PCS_33_90S_70L







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PROJECT NO. <u>178P.5.R.77</u> WARREN COUNTY 15+02.50 -L-STATION: _

SHEET 4 OF 6

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

SPAN C PLAN OF 25' UNIT 90° SKEW

BY: DATE: NO. BY: DATE: S-9	ı
3 TOTAL SHEETS	
22	

DRAWN BY:

E. PHELPS

J. LOFTUS

ELASTOMERIC BEARING DETAILS

(TYPE I - 66 REQ'D)

ELASTOMER IN ALL BEARINGS SHALL BE 60 DUROMETER HARDNESS.

DEAD LOAD DEFLECTION AT	ND CAMBER
	3'-0" × 2'-0"
25' CORED SLAB UNIT	0.6″∅ L.R. STRAND
CAMBER (SLAB ALONE IN PLACE)	1/4"
DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD**	0″ ♦
FINAL CAMBER	1/4"

** INCLUDES FUTURE WEARING SURFACE

DEAD LOAD DEFLECTION AN	ND CAMBER
	3'-0" × 2'-0"
70' CORED SLAB UNIT	0.6″∅ L.R. Strand
CAMBER (SLAB ALONE IN PLACE)	21/4"
DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD**	3/4″ ♥
FINAL CAMBER	11/2″ ♣
W THELLINES FLITHDE WEADTHE CHOI	

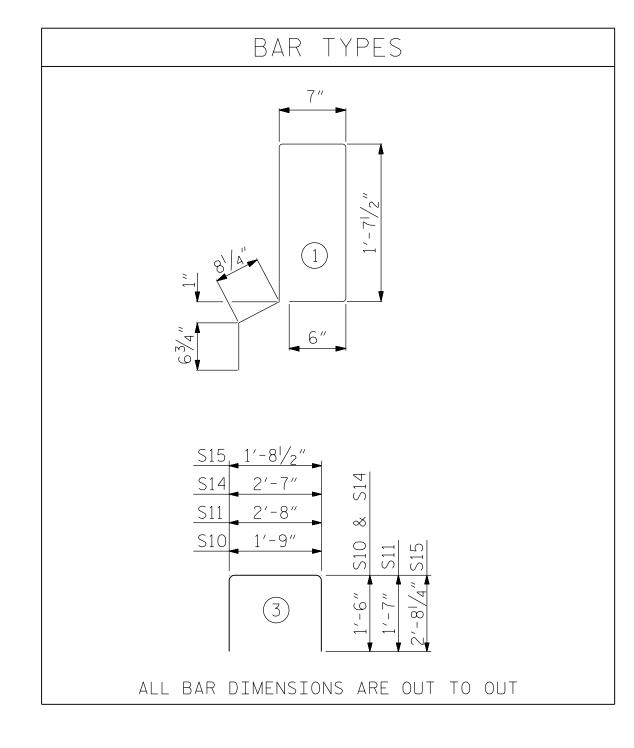
** INCLUDES FUTURE WEARING SURFACE

	BILL OF MATERIAL FOR ONE 25' CORED SLAB UNIT											
				EXTERI	OR UNIT	INTERI	OR UNIT					
BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT	LENGTH	WEIGHT					
B21	2	#4	STR	24'-8"	33	24'-8"	33					
S10	8	#5	3	4'-9"	40	4'-9"	40					
S11	54	#4	3	5′-10″	210	5′-10″	210					
* S12	34	#5	1	5′-7″	198							
S14	4	#4	3	5′-7″	15	5′-7″	15					
S15	4	#5	3	7'-1"	30	7'-1"	30					
REINFO	ORCING S	STEEL	LBS	<u>.</u>	328		328					
	Y COATE Iforcind		l D	<u> </u>	198							
			LBS				1 E					
1000 1	P.S.I. CO	NUKETE	CU. YDS) a	4.5		4.5					
		ANDC	NI a	<u> </u>	Ω		0					
0.6"	L.R. STR	AND2	No) .	9		9					

			OF NO'COF	MATERIA Red Slab		NE	
				EXTERI	OR UNIT	INTERI	OR UNIT
BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT	LENGTH	WEIGHT
B22	6	#4	STR	24'-6"	98	24'-6"	98
S10	8	#5	3	4'-9"	40	4'-9"	40
S11	144	#4	3	5′-10″	561	5′-10″	561
*S12	79	#5	1	5′-7″	460		
S14	4	#4	3	5′-7″	15	5′-7″	15
S15	4	#5	3	7'-1"	30	7'-1"	30
REINF(ORCING	STEEL	LB:	5.	744		744
	CY COATE						
	IFORCIN(460		
7000 F	P.S.I. CO	NCRETE	CU. YDS	<u>.</u>	11.8		11.8
0.6" Ø	L.R. STR	ANDS	No).	28		28

CONCRETE	RELE	ASE	STRENGTH
UNIT			PSI
25' UNITS			5500
70'UNITS			5500

GRADE 270 S	TRANDS
	0.6″Ø L.R.
AREA (SQUARE INCHES)	0.217
ULTIMATE STRENGTH (LBS.PER STRAND)	58,600
APPLIED PRESTRESS (LBS.PER STRAND)	43,950



CORED SLABS REQUIRED					
	NUMBER	LENGTH	TOTAL LENGTH		
25' UNIT					
EXTERIOR C.S.	2	25′-0″	50'-0"		
INTERIOR C.S.	9	25′-0″	225′-0″		
TOTAL	11	_	275′-0″		

COR	ED	SLABS	s req	UIRED
		NUMBER	LENGTH	TOTAL LENGTH
70' UNI	T			
EXTERIOR	C.S.	4	70'-0"	280'-0"
INTERIOR	C.S.	18	70'-0"	1260′-0″
TOTAL		22	_	1540′-0″

NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL CAST WITH THE CORED SLAB SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE CORED SLABS.

RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.

THE $2\frac{1}{2}$ \emptyset dowel holes at fixed ends of slab sections shall be FILLED WITH NON-SHRINK GROUT.

THE BACKER RODS SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER. SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.

WHEN CORED SLABS ARE CAST, AN INTERNAL HOLD-DOWN SYSTEM SHALL BE EMPLOYED TO PREVENT VOIDS FROM RISING OR MOVING SIDEWAYS. AT LEAST SIX WEEKS PRIOR TO CASTING CORED SLABS, THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR REVIEW AND COMMENT, DETAILED DRAWINGS OF THE PROPOSED HOLD-DOWN SYSTEM. IN ADDITION TO STRUCTURAL DETAILS, LOCATION AND SPACING OF THE HOLD-DOWNS SHALL BE INDICATED.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE CORED SLAB UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN THE REQUIRED STRENGTH SHOWN IN THE "CONCRETE RELEASE STRENGTH" TABLE.

ALL REINFORCING STEEL IN VERTICAL CONCRETE BARRIER RAILS SHALL BE EPOXY COATED.

PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE CORED SLAB UNIT ENDS.

APPLY EPOXY PROTECTIVE COATING TO CORED SLAB UNIT ENDS.

GROOVED CONTRACTION JOINTS, $\frac{1}{2}$ " IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

FLAME CUTTING OF THE TRANSVERSE POST-TENSIONING STRAND IS NOT ALLOWED.

MAINTAIN A SYMMETRIC TENSION FORCE BETWEEN EACH PAIR OF TRANSVERSE POST TENSIONING STRANDS IN THE DIAPHRAGM.

THE #4 S11 STIRRUPS MAY BE SHIFTED AS NECESSARY TO MAINTAIN 1" CLEAR TO THE GROUTED RECESS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

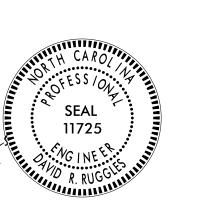
THE PERMITTED THREADED INSERTS ARE DETAILED AS AN OPTION FOR THE CONTRACTOR TO ATTACH FALSEWORK AND FORMWORK DURING CONSTRUCTION.

THE PERMITTED THREADED INSERTS IN THE EXTERIOR UNITS SHALL BE SIZED BY THE CONTRACTOR, SPACED AT 4'-0" CENTERS AND GALVANIZED IN ACCORDANCE WITH SECTION 1076 OF THE STANDARD SPECIFICATIONS. STAINLESS STEEL THREADED INSERTS MAY BE USED AS AN ALTERNATE.

THE PERMITTED THREADED INSERTS SHALL BE GROUTED BY THE CONTRACTOR IMMEDIATELY FOLLOWING REMOVAL OF THE FALSEWORK.

STATION: _

THE COST OF THE PERMITTED THREADED INSERTS SHALL BE INCLUDED IN THE PRICE BID FOR THE PRECAST UNITS.



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David Ruggle

7/17/2018

SHEET 5 OF 6 STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD

PROJECT NO. <u>178P.5.R.77</u>

WARREN

3'-0" X 2'-0" CORED SLAB UNIT

> SHEET NO REVISIONS S-10 NO. BY: DATE: DATE: TOTAL SHEETS

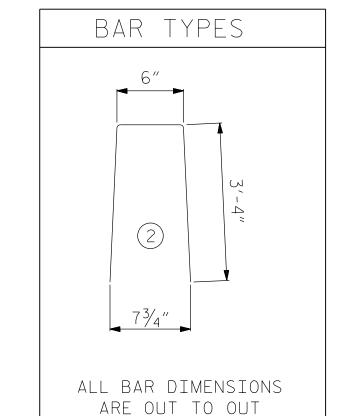
15+02.50 -L-

COUNTY

E. PHELPS DRAWN BY: DATE : 2/18 DATE : ___2/18 J. LOFTUS DESIGN ENGINEER OF RECORD: __J.LOFTUS__ DATE : __2/18_

ALL REINFORCING STEEL IN VERTICAL CONCRETE BARRIER RAILS SHALL BE EPOXY COATED.

GROOVED CONTRACTION JOINTS, $\frac{1}{2}$ " IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.



B.	AR TY
	6"
	2
	73/.″

BILL OF MATERIAL FOR VERTICAL CONCRETE BARRIER RAIL						
BAR	BARS PER PAIR OF EXTERIOR UNITS	TOTAL NO.	SIZE	TYPE	LENGTH	WEIGHT
	25' UNIT					
 ₩B26	20	20	#5	STR	24'-8"	515
* S13	68	68	#5	2	7'-2"	508
* EPOXY COATED REINFORCING STEEL LBS.						1023
CLASS AA CONCRETE CU.YDS.					3	6.7
TOTAL VERTICAL CONCRETE BARRIER RAIL LN. F						50.125

GUTTERLINE ASPHALT THICKNESS & RAIL HEIGHT

25' UNITS

70'UNITS

ASPHALT OVERLAY THICKNESS @ MID-SPAN

31/4"

2"

RAIL HEIGHT @ MID-SPAN

3'-9¹/₄"

3'-8"

BILL OF MATERIAL FOR VERTICAL CONCRETE BARRIER RAIL						
BAR	BARS PER PAIR OF EXTERIOR UNITS	TOTAL NO.	SIZE	TYPE	LENGTH	WEIGHT
	70'UNIT					
 ₩B25	60	120	#5	STR	22'-11"	2868
* S13	158	316	#5	2	7'-2"	2362
* EPOXY COATED REINFORCING STEEL LBS. 52						
CLASS AA CONCRETE CU.YDS.						36.2
TOTAL	TOTAL VERTICAL CONCRETE BARRIER RAIL LN.FT. 28					

David Ruggle 7/17/2018

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

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PROJECT NO. __17BP.5.R.77

15+02.50 -L-STATION: __

WARREN

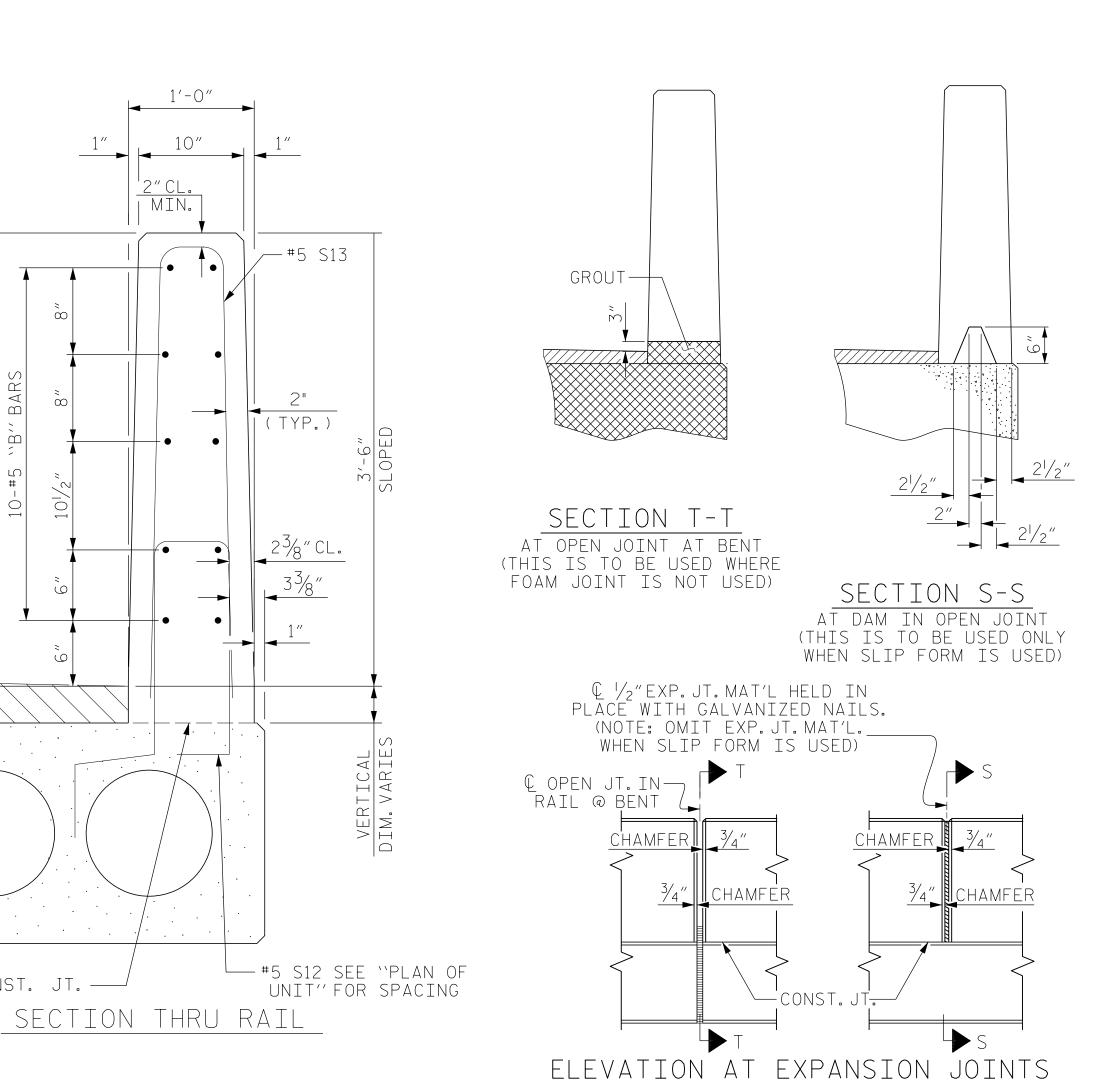
SHEET 6 OF 6

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

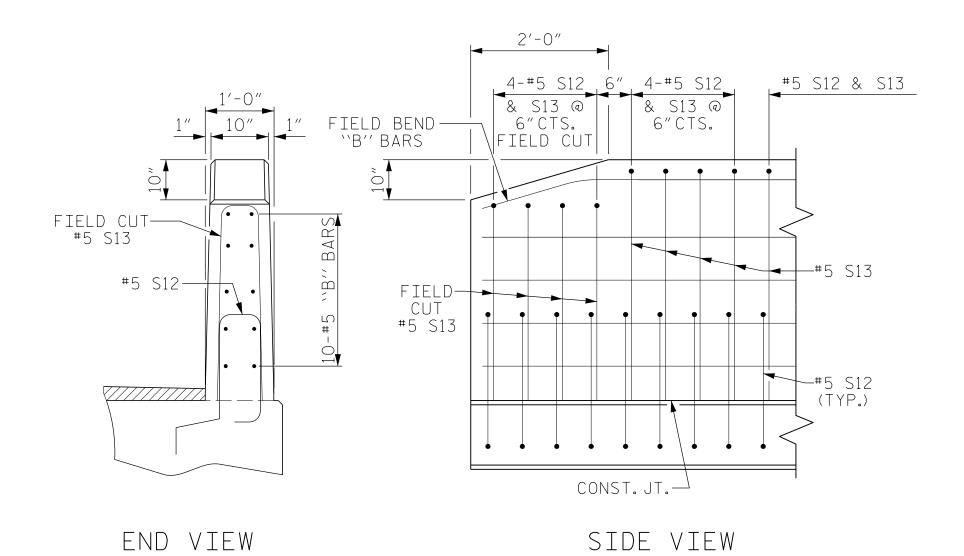
COUNTY

VERTICAL CONCRETE BARRIER RAIL DETAILS

SHEET NO REVISIONS S-11 NO. BY: DATE: DATE: TOTAL SHEETS 22



VERTICAL CONCRETE BARRIER RAIL DETAILS



END OF RAIL DETAILS

E. PHELPS DATE : 2/18 DRAWN BY: DATE : 2/18 J. LOFTUS

10"

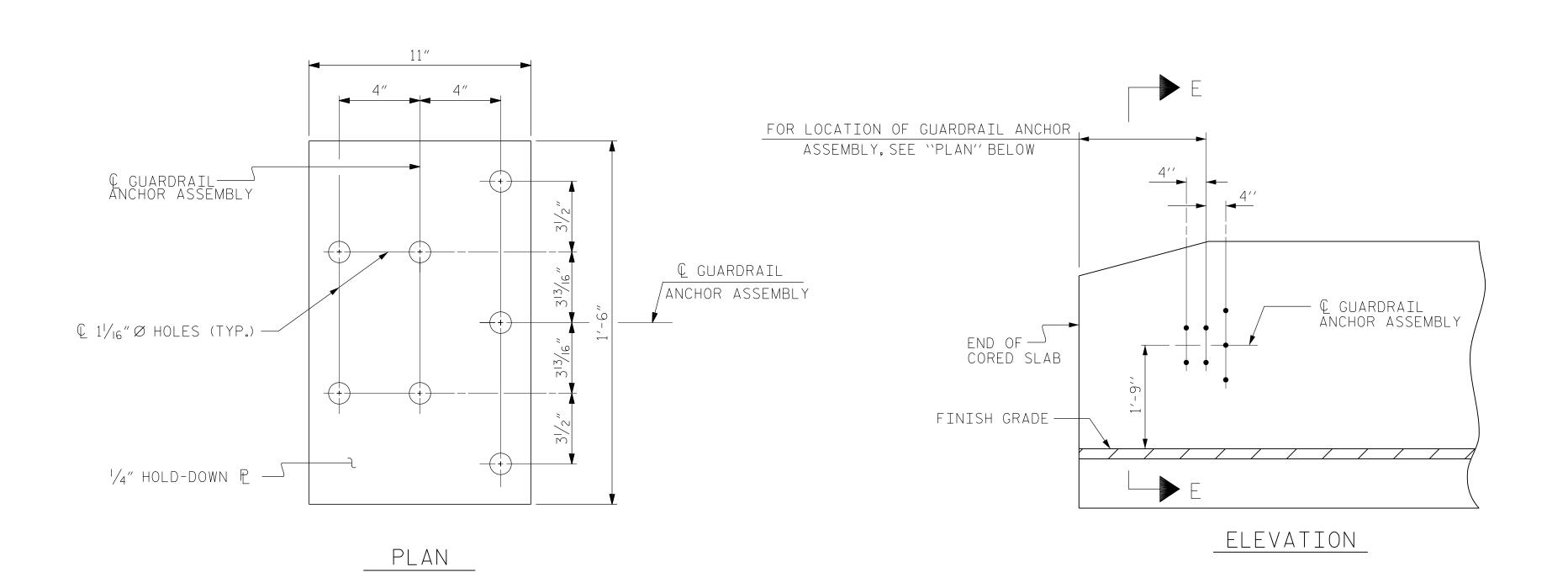
3'-9½" GUTTERLINE ASPHALT RAIL HEIGHT" TABLE)

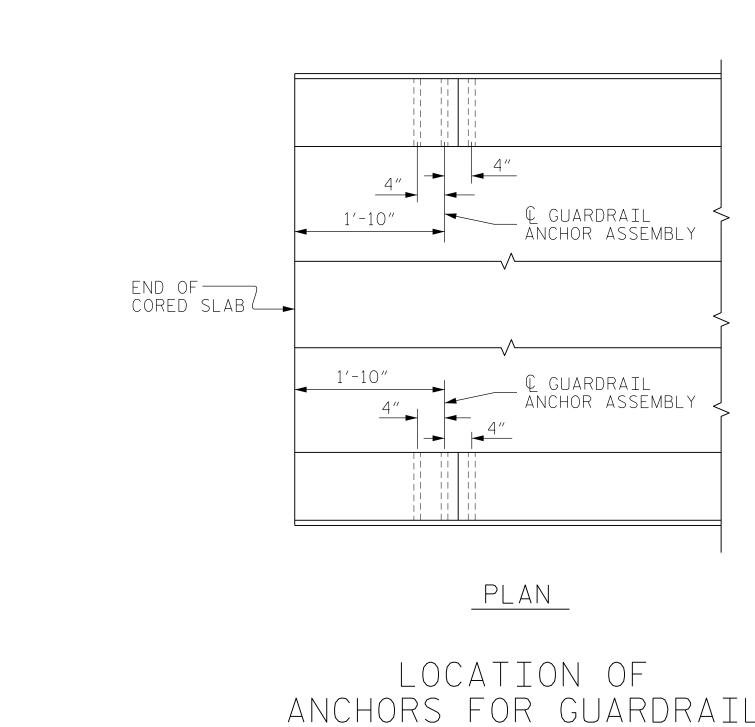
VARIES (SEE 'THICKNESS &

10-

10/

CONST. JT. —





END BENT #1 SHOWN, END BENT #2 SIMILAR.

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A 1/4" HOLD DOWN PLATE AND 7 - 7/8" Ø BOLTS WITH NUTS AND WASHERS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36.AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE \(\frac{7}{8}'' \omega \) GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

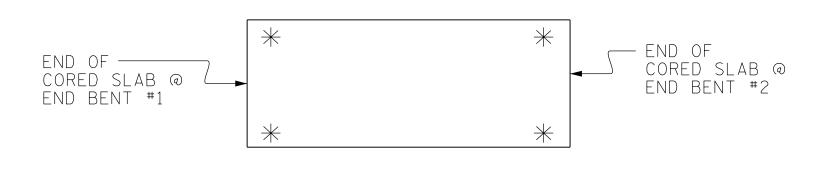
THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL.FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR VERTICAL CONCRETE BARRIER RAIL.

THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE VERTICAL CONCRETE BARRIER RAIL TO CLEAR ASSEMBLY BOLTS.

THE 1 1/4" Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.



SKETCH SHOWING POINTS OF ATTACHMENT

* DENOTES GUARDRAIL ANCHOR ASSEMBLY

PROJECT NO. 17BP.5.R.77

WARREN COUNTY

STATION: 15+02.50 -L-



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DEPARTMENT OF TRANSPORTATION

STANDARD

STANDARD

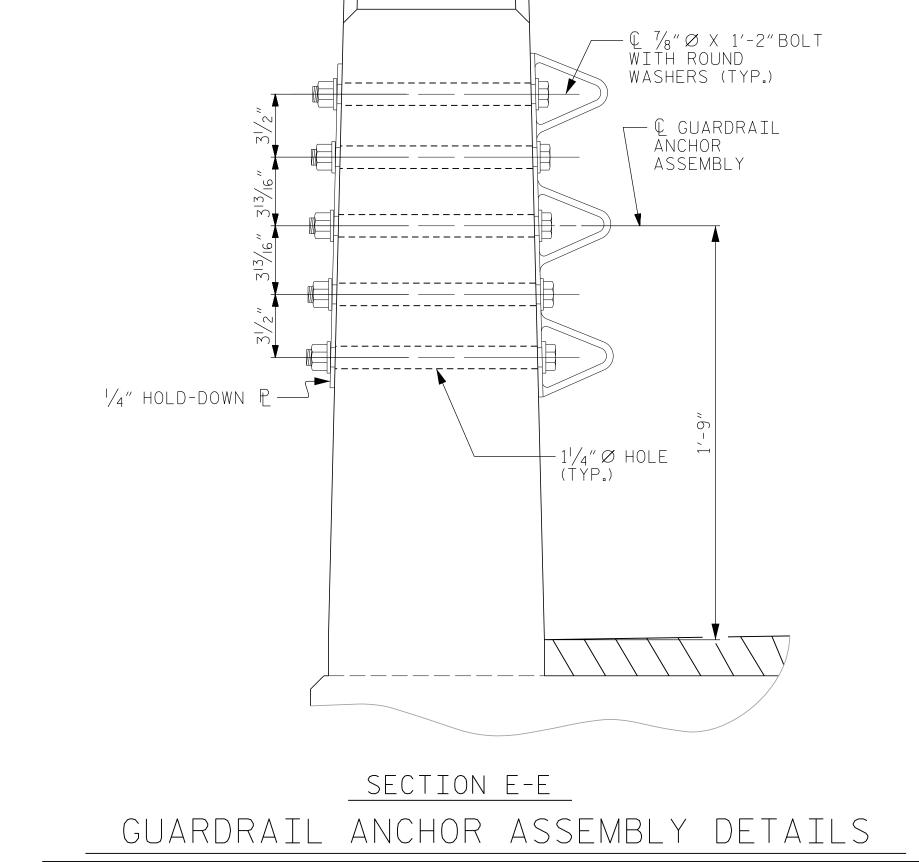
GUARDRAIL ANCHORAGE

DETAILS

FOR VERTICAL CONCRETION

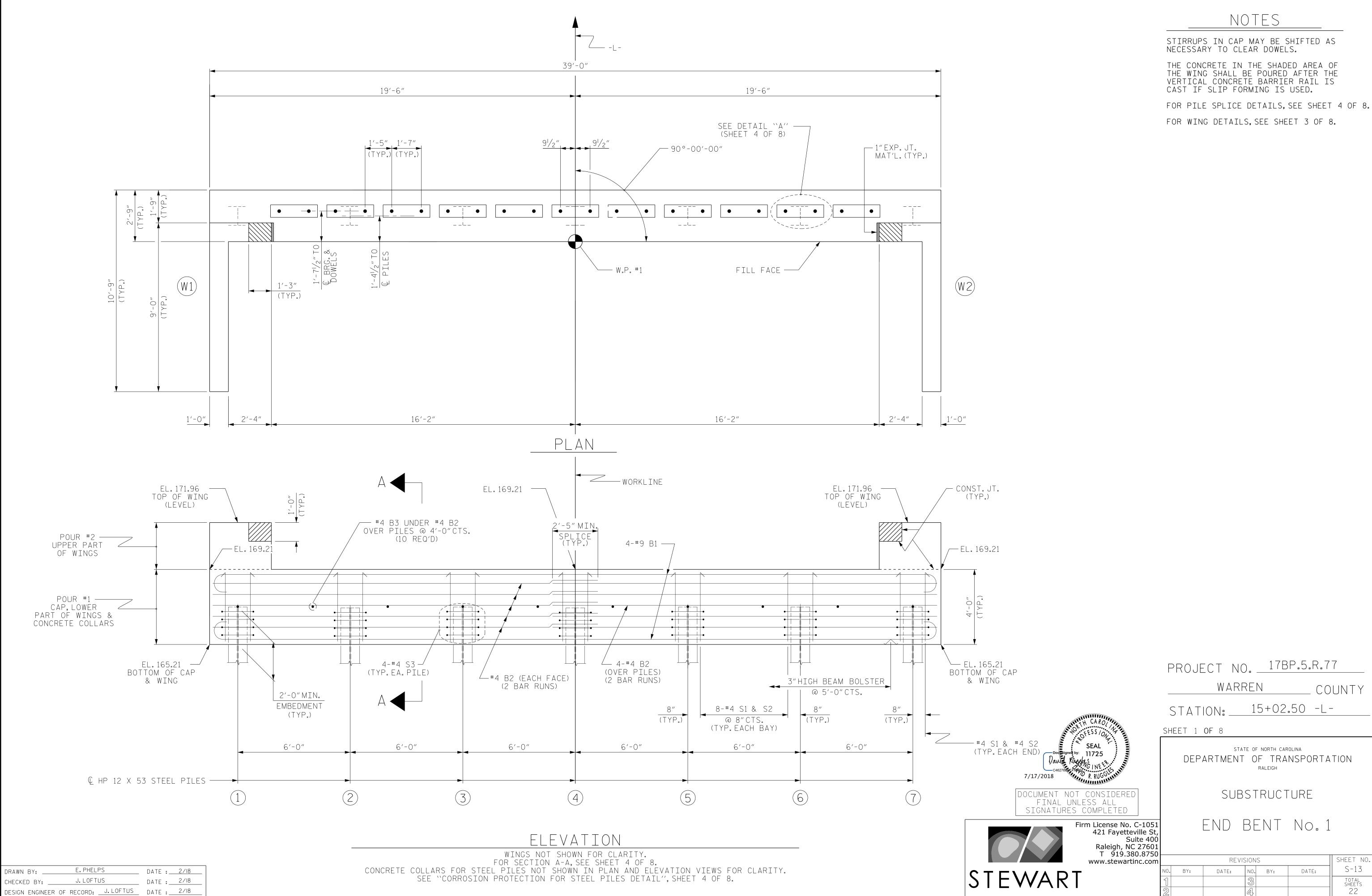
BARRIER RAIL

REVISIONS			SHEET NO.	
DATE:	NO.	BY:	DATE:	S-12
	3			TOTAL SHEETS
	4			22



(SHT 1b) STD.NO.GRA3

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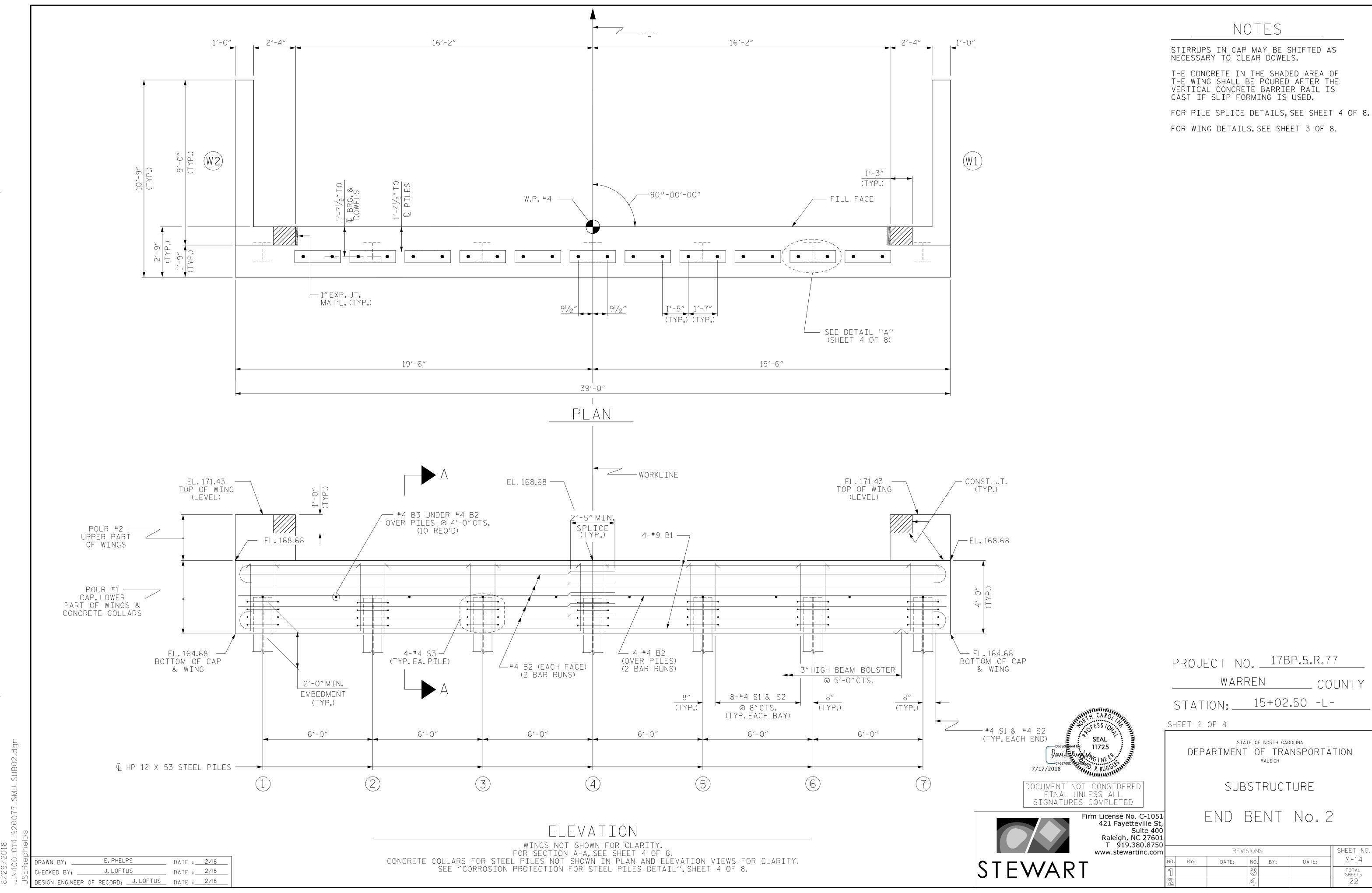


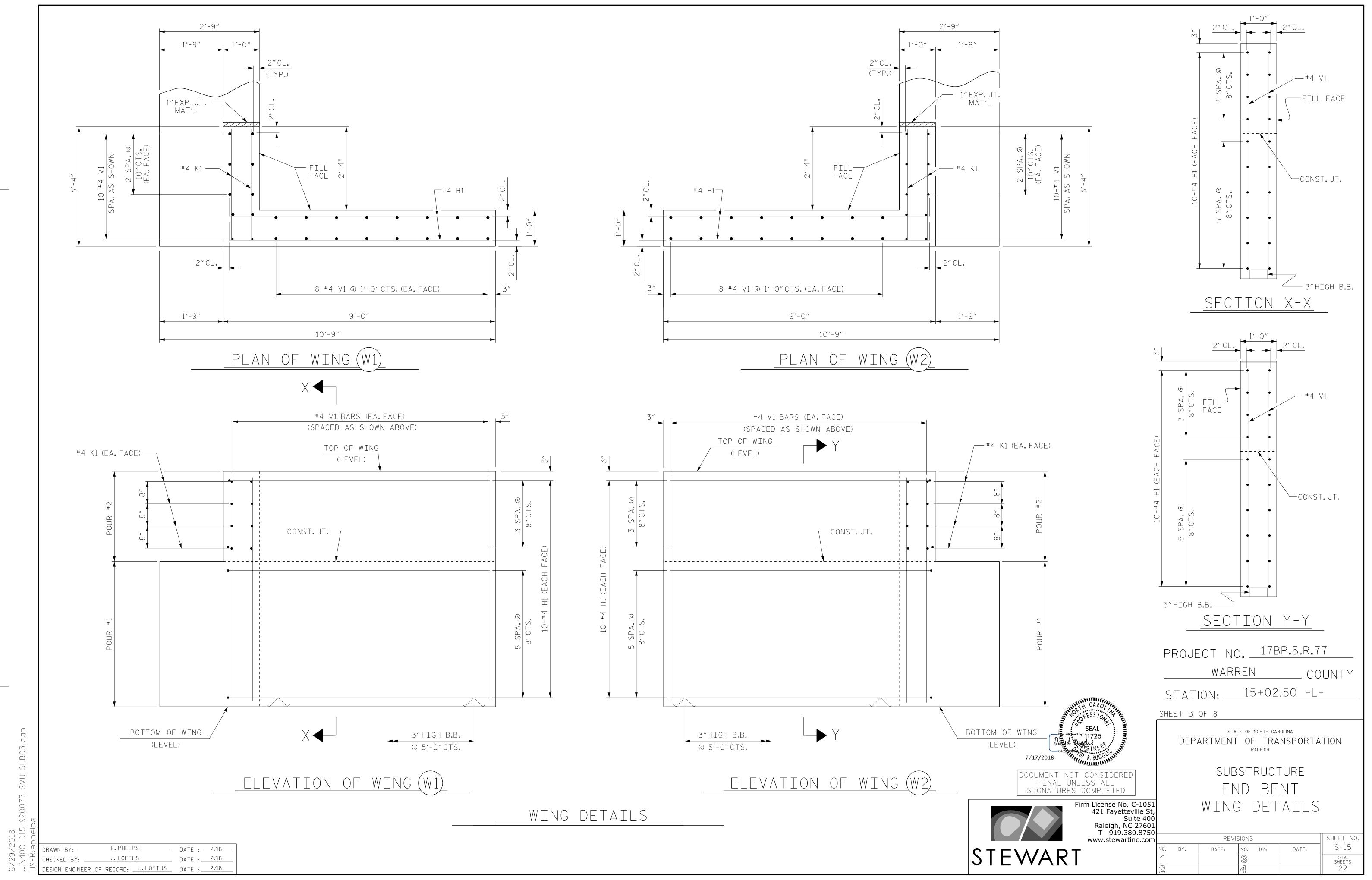
COUNTY

SHEET NO

S-13

TOTAL SHEETS 22





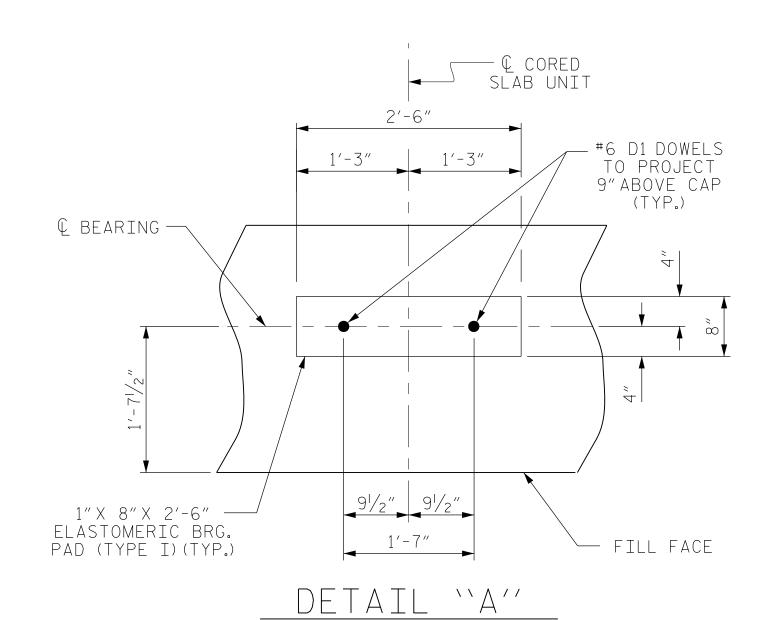
STD.NO.EB_33_90S4

BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

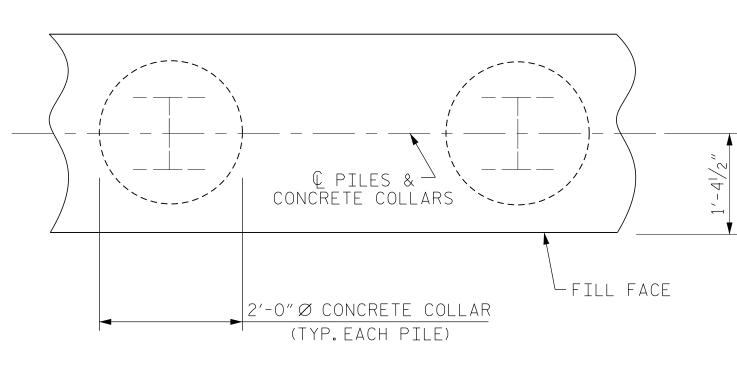
BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETER-MINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

TEMPORARY DRAINAGE AT END BENT



(END BENT No. 1 SHOWN, END BENT No. 2 SIMILAR BY ROTATION)



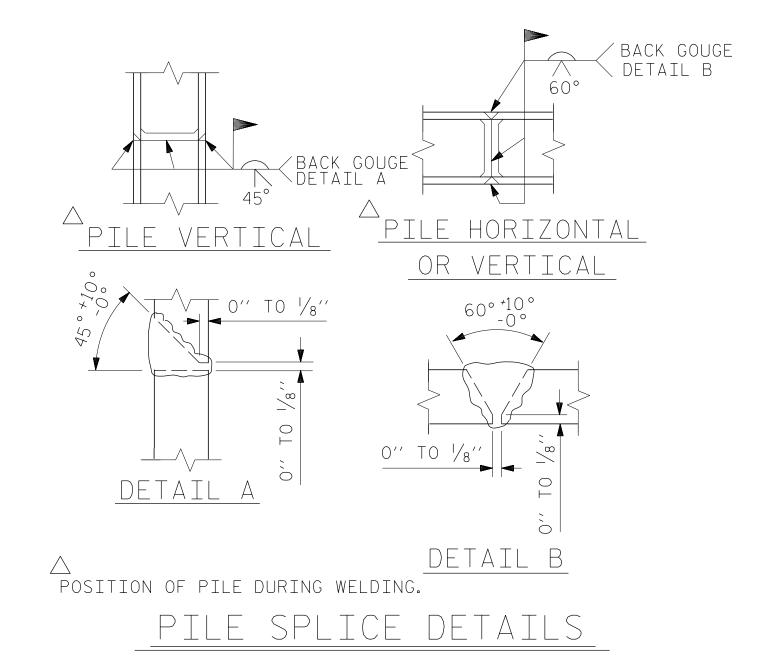
CORROSION PROTECTION FOR STEEL PILES DETAIL

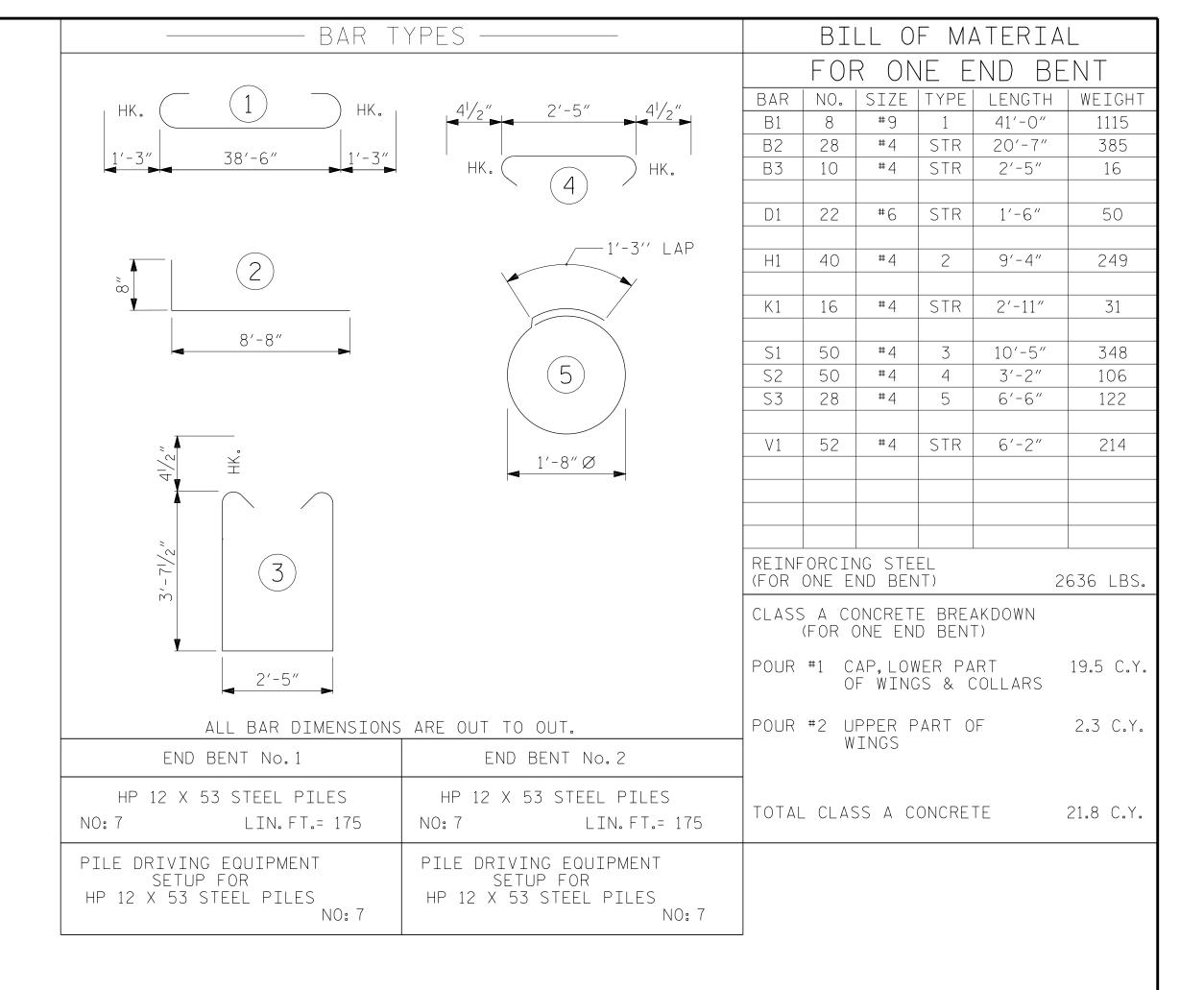
CONCRETE —

ELEVATION

COLLAR

(END BENT No. 1 SHOWN, END BENT No. 2 SIMILAR BY ROTATION)



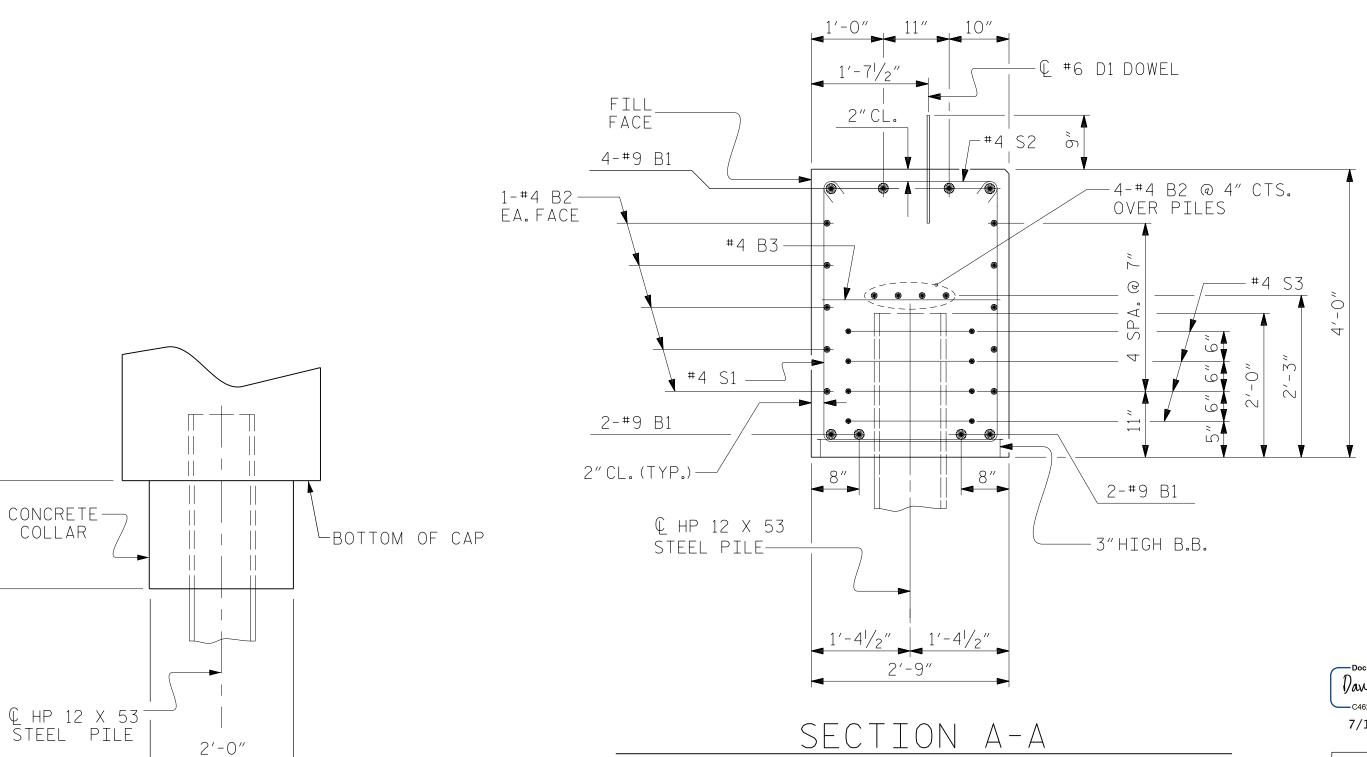


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PROJECT NO. <u>17BP.5.R.77</u> WARREN COUNTY 15+02.50 -L-STATION: _ SHEET 4 OF 8 STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

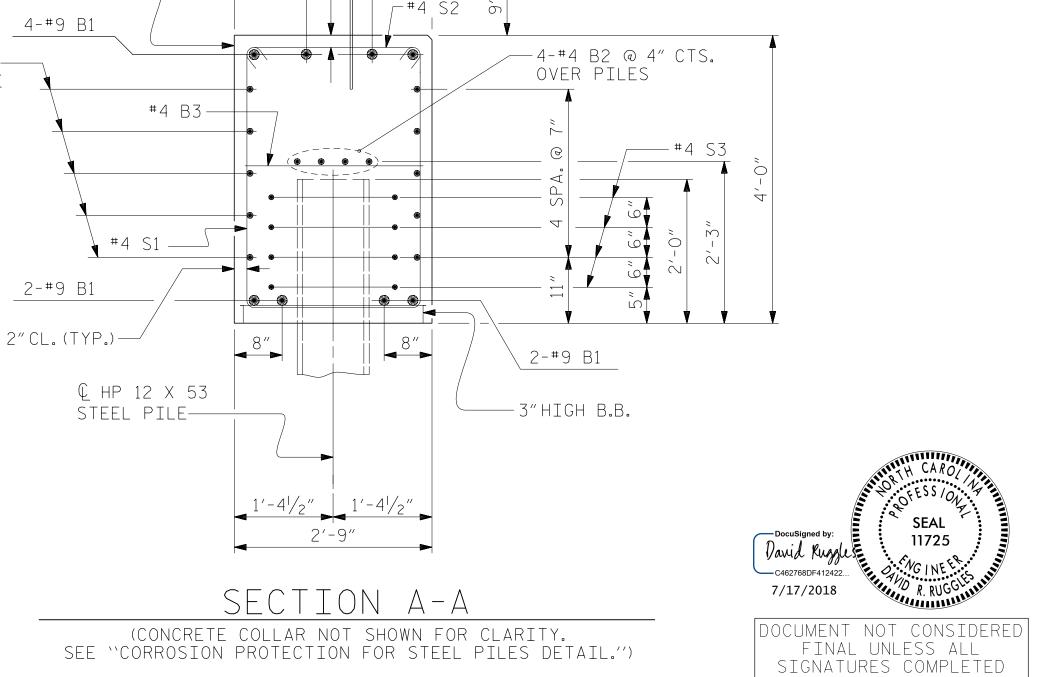
SUBSTRUCTURE

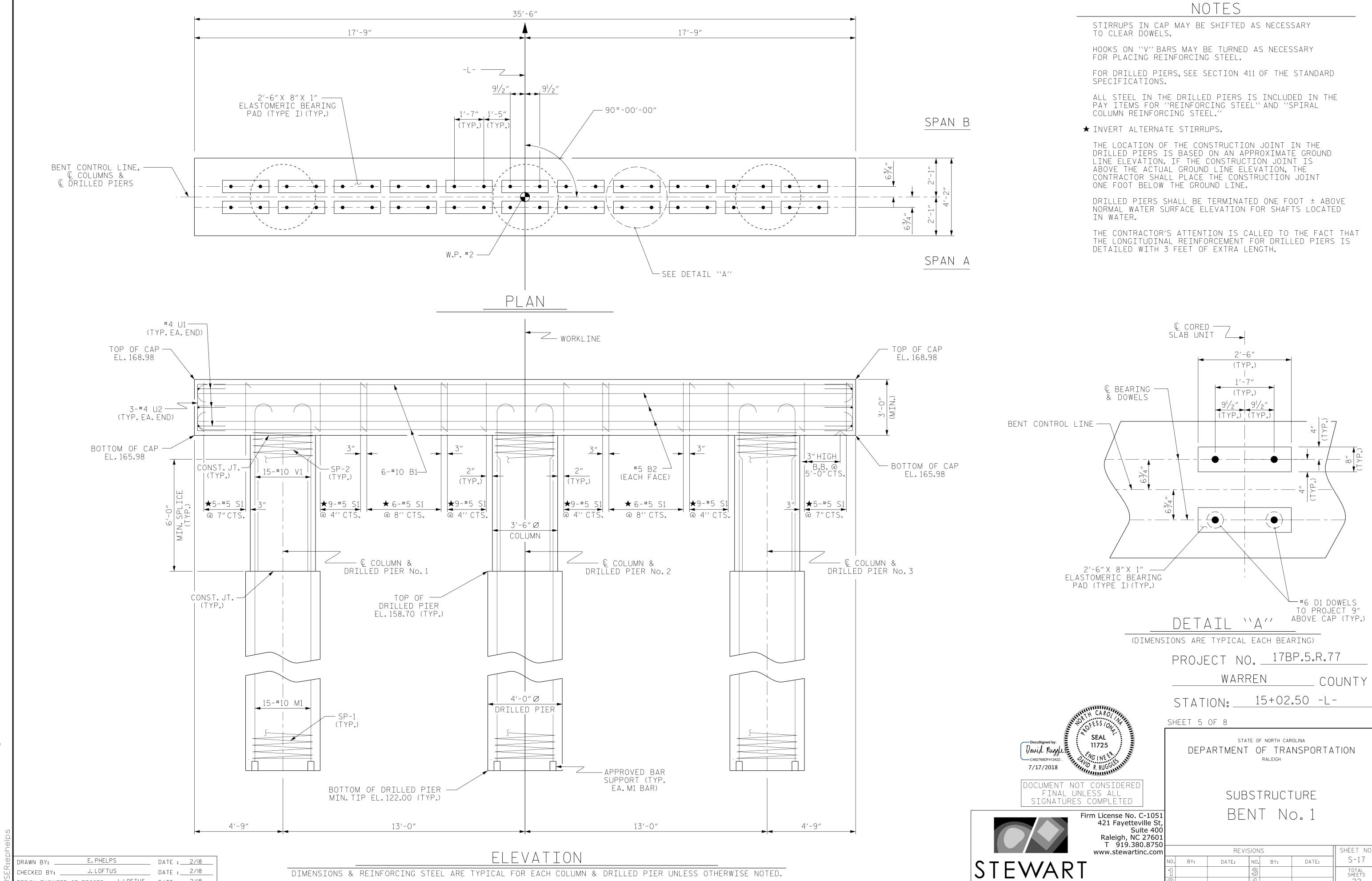
END BENT No.1 & 2 DETAILS

SHEET NO REVISIONS S-16 NO. BY: DATE: DATE: TOTAL SHEETS

PLAN E. PHELPS DRAWN BY: J. LOFTUS

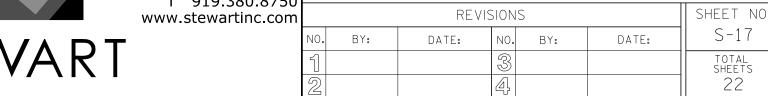
DATE : 2/18 DATE : 2/18 DESIGN ENGINEER OF RECORD: __J.LOFTUS__ DATE : ___2/18_

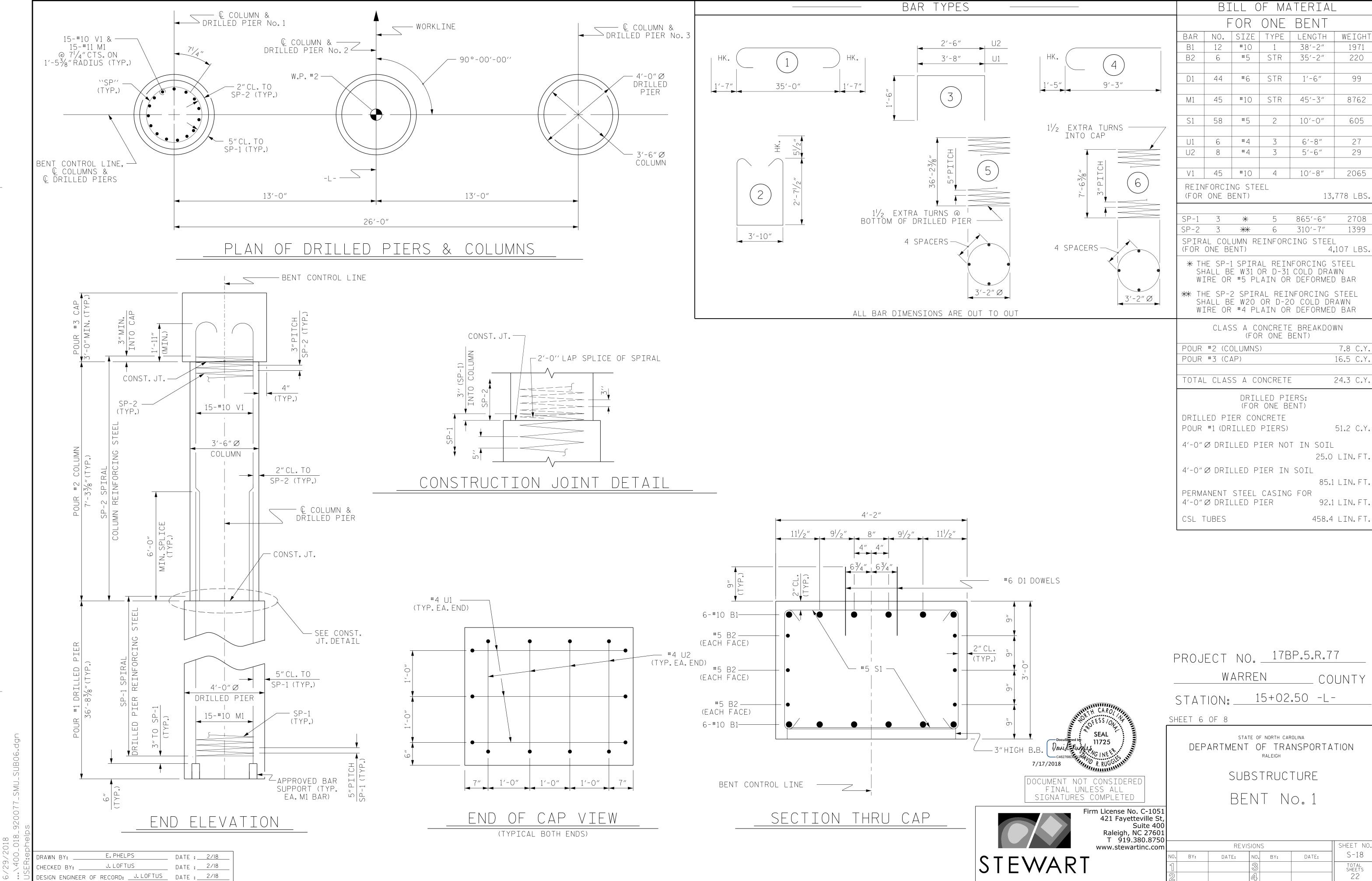


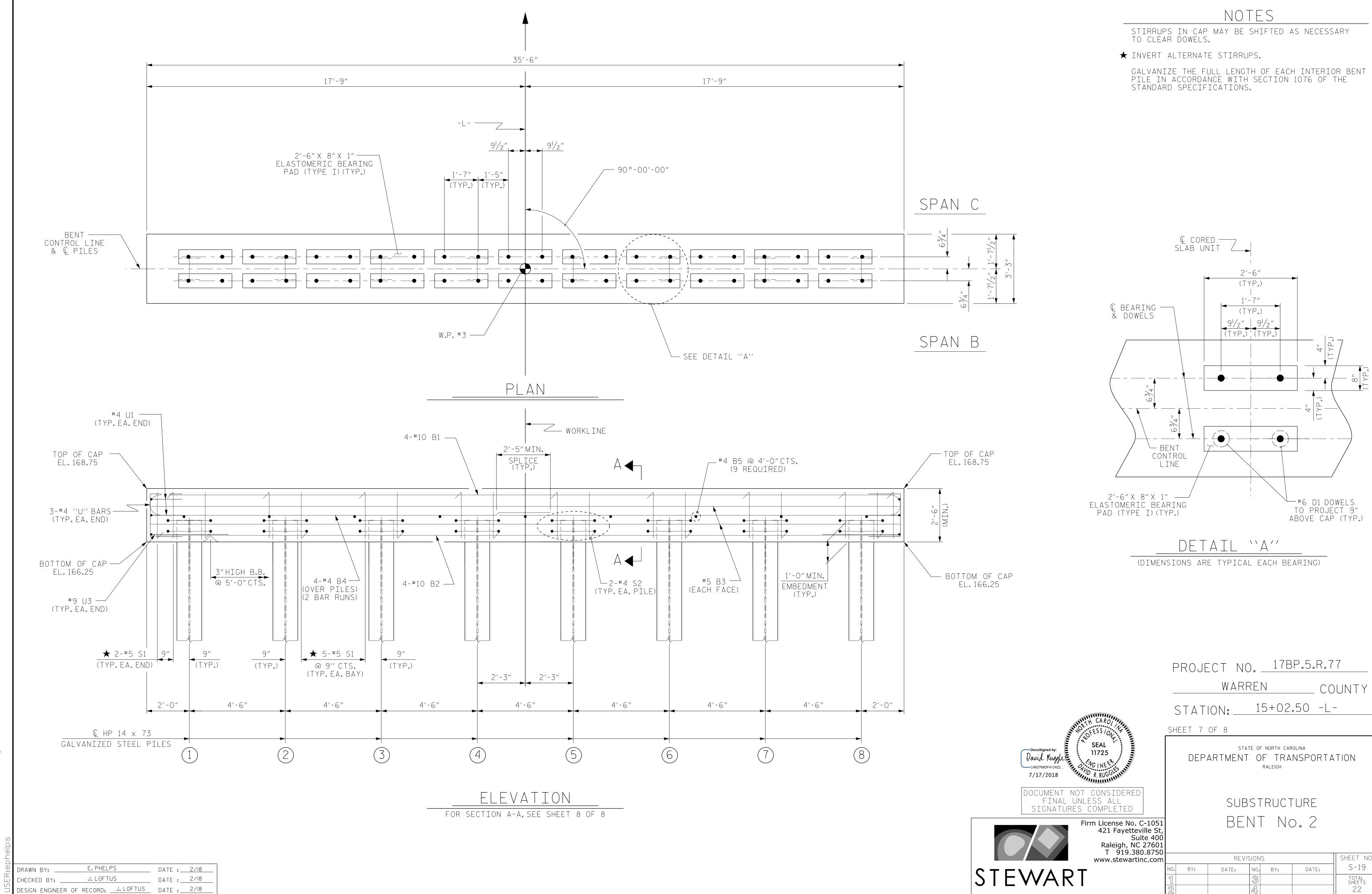


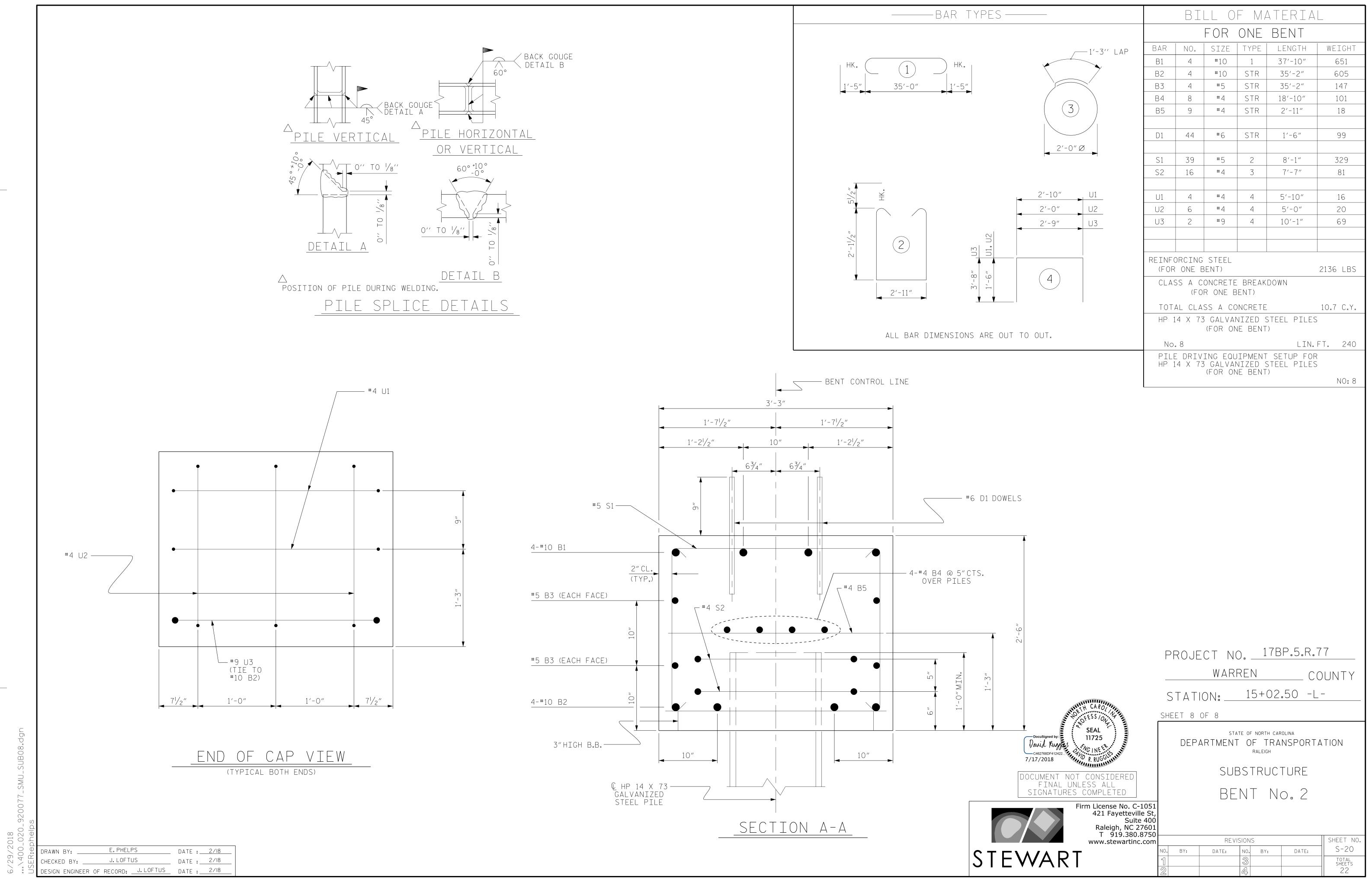
DESIGN ENGINEER OF RECORD: __J.LOFTUS__ DATE : ___2/18_

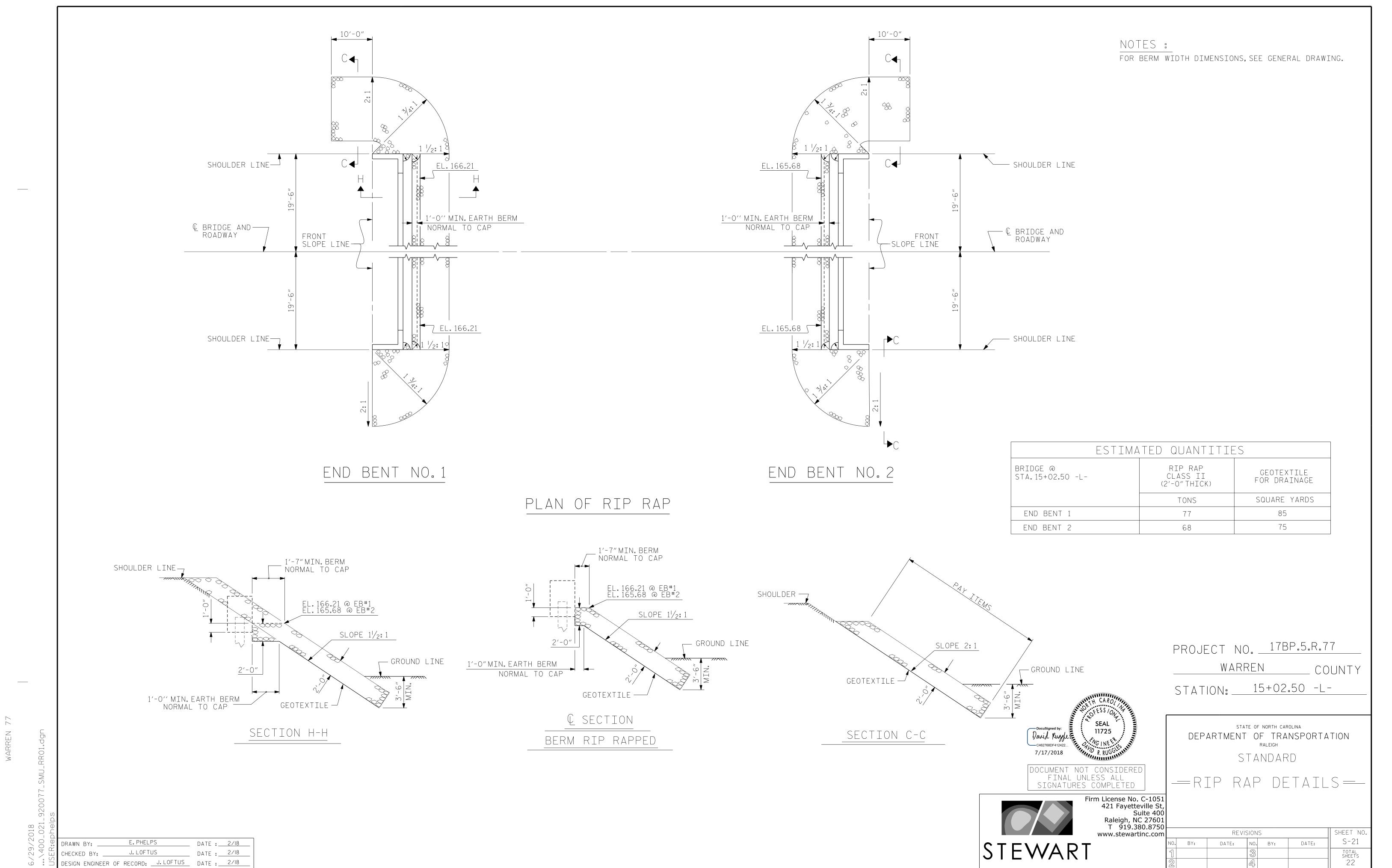
DIMENSIONS & REINFORCING STEEL ARE TYPICAL FOR EACH COLUMN & DRILLED PIER UNLESS OTHERWISE NOTED.



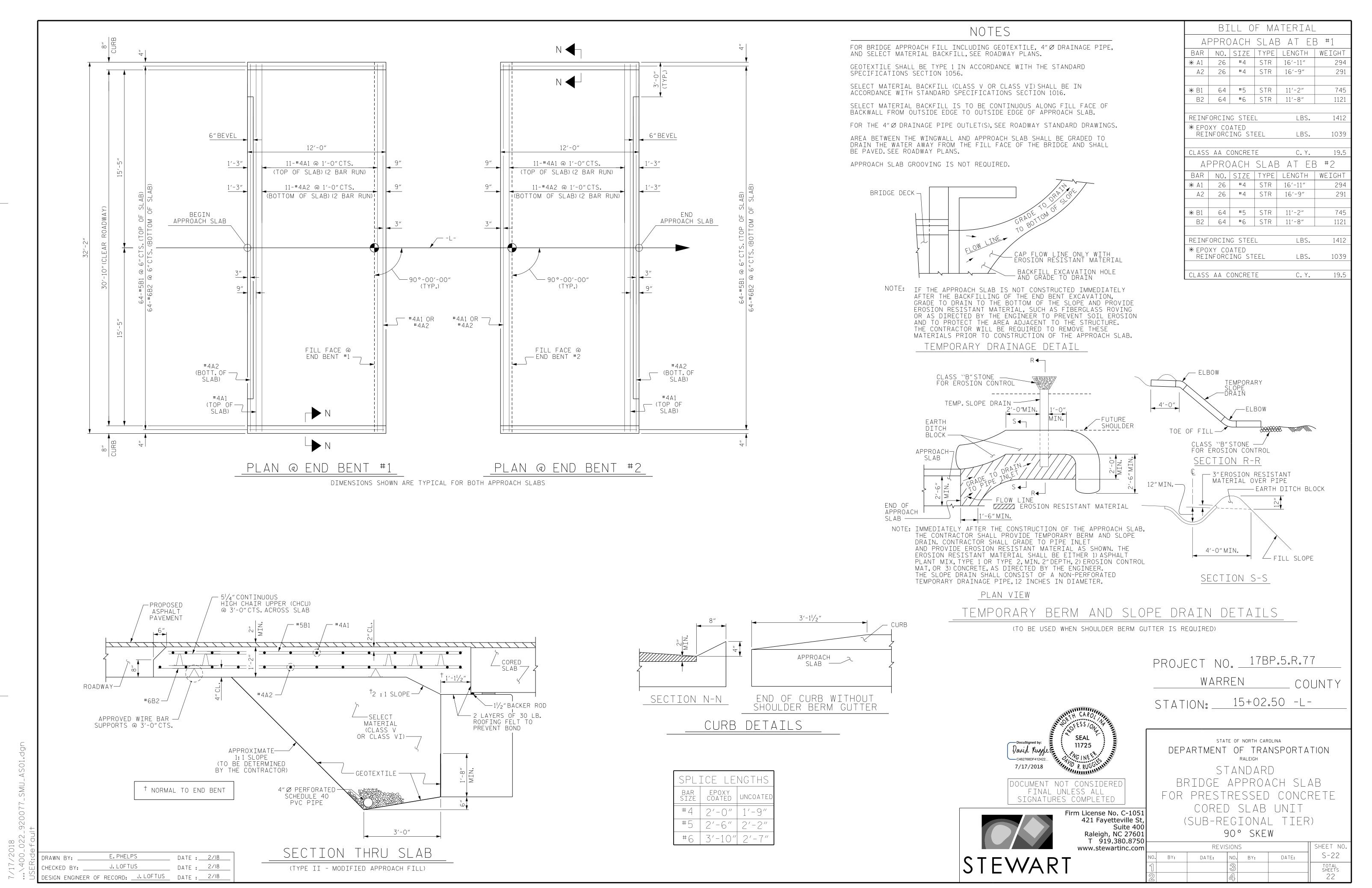








STD. NO. RR1 (Sht 2)



STANDARD NOTES

DESIGN DATA:

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2018 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N.C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED \(^3\)_4"WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1\(^2\)_2"RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A \(^4\)_4"FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A \(^4\)_4"RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE $\frac{7}{8}$ " Ø SHEAR STUDS FOR THE $\frac{3}{4}$ " Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - $\frac{7}{8}$ " Ø STUDS FOR 4 - $\frac{3}{4}$ " Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF $\frac{7}{8}$ " Ø STUDS ALONG THE BEAM AS SHOWN FOR $\frac{3}{4}$ " Ø STUDS BASED ON THE RATIO OF 3 - $\frac{7}{8}$ " Ø STUDS FOR 4 - $\frac{3}{4}$ " Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2" OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/6 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

ENGLISH